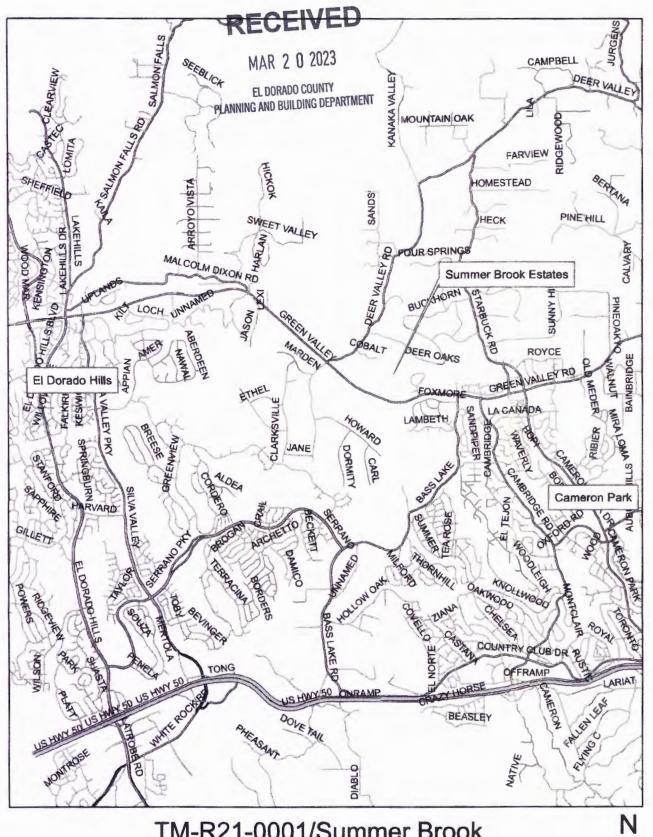
EL DORADO COUNTY PLANNING DEPARTMENT APPLICATION FOR Map Amendment for Parcel & Subdivision Maps ASSESSOR'S PARCEL NO.(s) 102-210-12 & 13 PROJECT NAME/REQUEST: (Describe proposed use) Orginal name Summerbrook Estates/rename Oak Haven No change in use. IF SUBDIVISION/PARCEL MAP: Create 29 existin lots, ranging in size from 58,591s ft to 91,113sqft acre(s) / SF IF ZONE CHANGE: From 11/2 to IF GENERAL PLAN AMENDMENT: From 11/4 to IF TIME EXTENSION, REVISION, CORRECTION: Original approval date Expiration date APPLICANT/AGENT Blue Mountain Communities Mailing Address 707 Aldridge Road, Vacaville, CA 95688 Phone (707) 451-8111 x691 FAX (email) awong @bluemountaininc.n. PROPERTY OWNER Summerbrook El Dorado Inc. Mailing Address Same FAX() LIST ADDITIONAL PROPERTY OWNERS ON SEPARATE SHEET IF APPLICABLE ENGINEER/ARCHITECT CTA Bugineering & Surveying Mailing Address 3233 Monier Circle, Rancho Cordova, CA 95742 Phone (916) 638-0919 Green Valley Road North LOCATION: The property is located on the side of NIEIWIS Bass Lake Road feet/miles of the intersection with N/E/W/S major street or road Cameron Park <or pick from list> area. PROPERTY SIZE in the acreage / square footage Date signature of property owner or authorized agent FOR OFFICE USE ONLY Date 20 March 23 Fee \$ 2,117 Receipt # 45633 Rec'd by CEP Census Supervisor Dist 4 Sec/Twn/Rng Zoning RE-5-PD GPD LDR ACTION BY: PLANNING COMMISSION **ACTION BY BOARD OF SUPERVISORS ZONING ADMINISTRATOR** PLANNING DIRECTOR Hearing Date Approved Denied (findings and/or conditions attached) Hearing Date APPEAL: Approved Denied Approved Denied (findings and/or conditions attached) **Executive Secretary** Executive Secretary Revised 07/02)

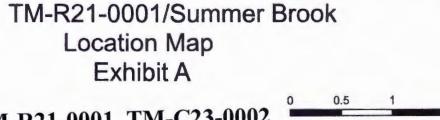
EL DORADO COUNTY PLANNING DEPARTMENT

REQUIRED SUBMITTAL INFORMATION for Map Amendment for Parcel & Subdivision Maps

The following information must be provided with all applications. If all the information is not provided, the application will be deemed incomplete and will not be accepted. For your convenience, please use the check ($\sqrt{}$) column on the left to be sure you have all the required information. All plans and maps MUST be folded to 8½" x 11".

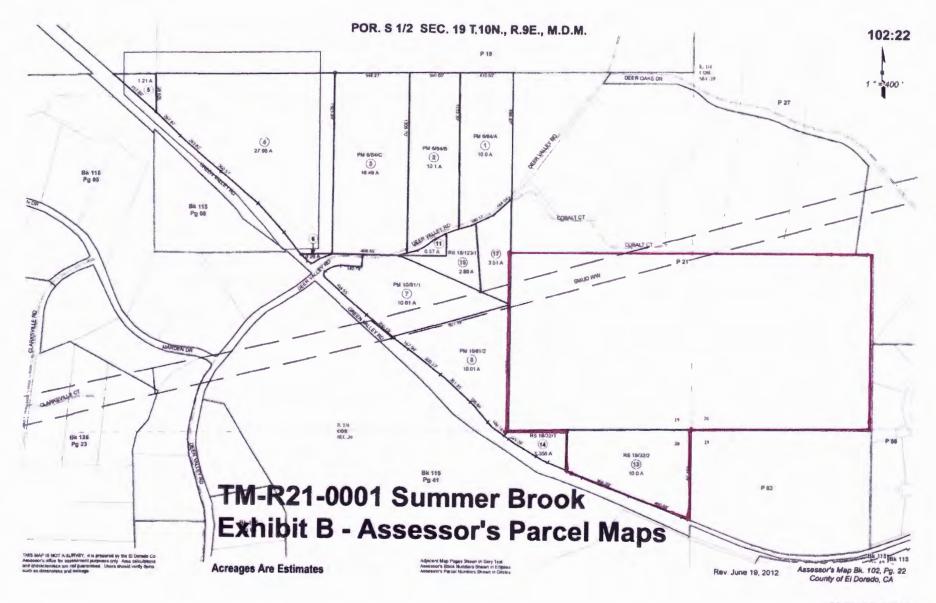
| Check (\(\sigma\) Applicant Cou |) | SREQUIRED |
|---------------------------------|----|--|
| | 1) | Application Form and Agreement for Payment of Processing Fees, completed and signed. |
| N/A | 2) | Letter of authorization from <u>all</u> property owners authorizing agent to act as applicant, when applicable. |
| N/A | 3) | Proof of ownership (Grant Deed), if the property has changed title since the last tax roll. |
| | 4) | A copy of official Assessor's map, showing the property outlined in red. |
| | 5) | An 8 $\frac{1}{2}$ x 11° vicinity map showing the location of the project in relation to the distance to major roads, intersections, and town sites. |
| | 6) | Provide name, mailing address and phone number of all property owners and the agents. |
| N/A | 7) | Ten (10) copies of the proposed map amendment, folded to 81/2" x 11". |
| | 8) | Narrative of requested amendment, including supporting documentation, if applicable. |
| N/A | 9) | Name and address of Homeowners' Association, CSA 9 Zone of Benefit, or other road maintenance entity if it exists in the project area. |





22-0616 D 1 of 49

TM-R21-0001, TM-C23-0002



Proposed Amendment to Project Conditions

- 25. The applicant shall signalize construct street lighting to illuminate the Green Valley/Deer Valley Road intersection to meet current El Dorado County Standards, as required in the approved traffic study. These required enhancements shall include street lighting with the use of cobra head LED fixtures in accordance with County Standards and the provision of electrical power to the light standards. Subdivider shall provide a funding source e.g. inclusion in the project Home Owners Association (HOA) budget to cover the cost of electricity and general maintenance of the equipment geometric improvements to Green Valley Road consistent with the approved improvement plans for CIP Project no. 6614 which includes the intersection widening to provide for right and left turn channelization and acceleration/deceleration lanes and shall adhere to the latest version of the Manual Uniform Traffic Control Devices (MUTCD), the California Supplement, and the Caltrans Highway Design Manual. The improvements shall be substantially completed to the approval of the Department of Transportation or the applicant shall obtain an approved revised Improvement Agreement with security reflecting the changes above, prior to the filing the final map for Phase 3 of the Subdivision.
- 26. The signal controller and controller cabinets shall be approved the Department of Transportation Operations Maintenance prior to purchase of said items.

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PLANNING AND BUILDING DEPARTMENT

Narrative to Application for Amended Final Map

This application seeks the amendment of Condition No. 25 of the tentative map approved by the County in 2008, which was subsequently incorporated into the final map approved by the Board of Supervisors on October 11, 2022, requiring that the project be responsible for the construction of traffic signals at the intersection of Green Valley Road and Deer Valley Road (the "Subject Intersection"). All other Conditions have been satisfied, including intersection improvements, construction of a Class 2 bike lane and irrevocable offers of dedication of right of way for Green Valley Road slope easements.

The basis for this request is that there are changed circumstances that make the installation of traffic signals at the Subject Intersection no longer appropriate or necessary. In 2007 and 2008, when the traffic analysis was performed for the project, the traffic engineers were utilizing an early iteration of the County Traffic Demand Model which resulted in a finding that Subject Intersection was failing with or without the addition of traffic from the project. Since the project would add more than 10 vehicle trips in the a.m. peak hour, it was deemed to "worsen" the existing deficiency pursuant to General Plan Policies TC-Xa and TC-Xe. The determination that the project would worsen the impacts resulted in the application of Policy TC-Xf requiring mitigation. The traffic studies provided that to reduce impacts in compliance, with applicable standards, would require extensive physical improvement to the Subject Intersection, including signalization. These requirements were embodied in Conditions 25, 26 and 27 of the tentative map.

Since the approval of the project the County undertook an extensive updating of its Traffic Demand Model and Capital Improvement Plan. It was determined through the update process that some of the assumptions on population growth and densities utilized in the earlier model were dramatically overstated and a new Transportation Demand Model was adopted utilizing updated assumptions. Further, significant improvements were made at the Subject Intersection consistent with the physical improvements contained in the project Conditions 26 and 27.

Updated traffic reports were prepared for the project by Kimley Horn (the traffic engineer that conducted the study in 2007) in 2019, 2021 and 2022, which reveal that the Subject Intersection is operating well within County standards today and is projected to remain in compliance in future cumulative conditions. The updated project traffic studies revealed that the baseline traffic volumes utilized in the original analysis for 2007, is higher than the measured volumes in 2021, and 2022. As part of the recent traffic study updates a traffic signal warrants analysis was performed which concluded that warrants for signals were not met for existing or future traffic volumes.

As a result of the updated traffic analysis and traffic signal warrants, DOT has indicated that signalization is unnecessary and undesirable. Understanding that the intent of the condition was to improve the performance of the Subject Intersection, other measures were discussed that could be implemented other than signals to enhance the performance or safety of the intersection. In accordance with those discussions, the Subdivider has agreed to install street lighting at the intersection consisting of pole lights with LED downcast light fixtures ("cobra heads") complying with County standards. In addition, a funding source will be provided for the cost of electricity and general maintenance of the lighting ensuring there is no financial burden to the County.

This application is consistent with Ordinance Code Sections 120.72.010 and meets the findings required by Section 120.72.040 to amend a final map, itemized below.

- 1. There are changes in circumstance which make the condition of the map no longer appropriate or necessary;
- 2. The proposed modification does not impose any additional burden on the present fee owner of the property;
- 3. The modification proposed does not alter any right, title or interest in the real property reflected in the recorded map;
- 4. The map as modified conforms to the provisions of Government Code Section 66474.

Attachments

Kimley Horn, Green Valley Road Intersection Operations Analysis, April 12, 2022 Kimley Horn, Traffic Signal Warrant Evaluation-Green Valley Road at Deer Valley Road, June 4, 2021

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EL DORADO COUNTY
PLANNING AND BUILDING DEPARTMENT

Memorandum

To: Mike Harlan, Blue Mountain Communities

From: Robert Paderna, P.E., Kimley-Horn

Zachary Ramalingam, EIT, Kimley-Horn

Re: Summer Brook

Traffic Signal Warrant Evaluation - Green Valley Road at Deer Valley Road

Date: June 4, 2021

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EL DORADO COUNTY
PLANNING AND BUILDING DEPARTMEN

The purpose of this memorandum is to present the results of the traffic signal warrant evaluation completed for the Green Valley Road and Deer Valley Road intersection in El Dorado County, CA. We understand that El Dorado County has requested completion of the traffic signal warrant evaluation for the subject intersection as part of a condition of approval (COA) for the Summer Brook development project. This memorandum summarizes the signal warrant evaluation and our findings.

Data Collection

Kimley-Horn completed a site visit of the subject intersection on Monday, February 18, 2019, to observe posted speed limits, intersection lane configurations, and intersection geometry. Additionally, we collected 7-day, bi-directional roadway segment counts along each of the Green Valley Road and Deer Valley Road approaches to the subject intersection on the week of May 11, 2021. In addition, we collected intersection turning movement counts during the peak 8-hour window on May 26, 2021 for use in the traffic signal warrant evaluation. The traffic count data sheets are provided in **Attachment A**.

Traffic Control Warrants

We completed a traffic signal warrant evaluation for the subject intersection based on the methodologies noted in Section 4C.01 of the *California Manual on Uniform Traffic Control Devices (CMUTCD)*, 2014 Edition (with March 2018 revisions). The warrant evaluation was completed for the Existing plus Project traffic conditions and included the review of Warrants 1 through 4 noted below.

- Warrant 1 (Eight-Hour Vehicular Volume) is intended for application at locations where there is
 either a large volume of intersecting traffic (Condition A-Warrant 1A) or where the traffic volume
 on a major street is so heavy that the traffic on the minor intersecting street experiences delay or
 conflict in entering or crossing the major street (Condition B-Warrant 1B). The need for a traffic
 control signal shall be considered if an engineering study finds that one of the conditions
 (Condition A and Condition B) exist for each of any 8 hours of an average day.
- Warrant 2 (Four-Hour Vehicular Volume) is intended for locations where the volume of
 intersecting traffic is the principal reason to consider installing a traffic control signal. The need
 for a traffic control signal shall be considered if an engineering study finds that, for each of any 4
 hours of an average day the volume conditions are met.
- Warrant 3 (Peak Hour) is intended for use at a location where during any one (1) hour of an
 average day the minor street traffic experiences undue delay when entering or crossing the
 major street.
- Warrant 4 (Pedestrian Volumes) The Pedestrian Volume signal warrant is intended for application
 where the traffic volume on a major street is so heavy that pedestrians experience excessive
 delay in crossing the major street.



Based on the evaluation of the abovementioned traffic signal warrants, it was determined that the warrant for a traffic signal at the Green Valley Road at Deer Valley Road intersection is not satisfied under Existing plus Project conditions. The results of the warrant evaluation is summarized in **Table 1** below. Summary sheets for the warrant evaluation are included as **Attachment B**.

Table 1: Traffic Signal Warrant Evaluation Summary

| | | Traffic Sig | nal Warrant | |
|---|----------------------------|----------------------------|------------------------|--------------------------------|
| Intersection | Warrant 1 8-hour volume | Warrant 2 4-hour volume | Warrant 3 Peak Hour | Warrant 4 Pedestrian Volume |
| Green Valley Road & Deer Valley Road | Not Satisfied | Not Satisfied | Not Satisfied | Not Satisfied |

Equitable Share Responsibility

Because it was determined that the traffic signal warrant is not satisfied, the County has requested that the project applicant determine the project's fair share percentage of the construction costs to install a traffic signal at the subject intersection. The project's fair share contribution was calculated based on the project's proportionate traffic contribution to the overall future traffic volumes at the subject intersection. Based on the Caltrans *Guide for the Preparation of Traffic Impact Studies* (2002), the fair share calculation for impacts at an intersection is calculated using the following equation:

$$P = T/(T_B - T_E)$$

Where:

P = The equitable share for the proposed project's traffic impact.

T = The vehicle trips generated by the project during the peak hour of subject intersection in vehicles per hour, vph.

 T_B = The forecasted traffic volume on impacted intersection at the time of general plan build-out, vph.

 T_E = The existing traffic volume on the impacted intersection plus other approved projects that will generate traffic that has yet to be constructed/opened, vph.

Based on proposed project trips and traffic volumes contained in the *Summer Brook Traffic Impact Analysis* (dated February 14, 2007), we calculated a <u>fair share of 3.9%</u> for the proposed project's traffic impact.

Summary and Recommendations

Based on the traffic signal warrant criteria contained in the *CMUTCD*, the Warrants 1 through 4 are not satisfied at the Green Valley Road at Deer Valley Road intersection under Existing plus Project conditions. Therefore, no traffic control or other intersection control modifications are required at this time. The proposed project's fair share contribution percentage of construction costs to install a traffic signal was calculated to be 3.9%, which is equivalent to approximately \$10,000 to \$12,000. Note that this represents an opinion of probable construction costs under current market conditions, and Kimley-Horn cannot guarantee accuracy of construction cost estimates in future years due to factors beyond our control.

Attachments:

Attachment A – Traffic Counts

Attachment B – Warrant Analysis Worksheets



Attachment A
Traffic Counts

National Data & Surveying Services Intersection Turning Movement Count

| Control: 2 | tescue 2-Way Stop | | r Valley Rd | | | | | | | | | | Pn | Date: 5 | 21-070070-0 5/26/2021 | 001 | |
|--------------------|----------------------|--------------|-------------|---------|-------------|--------------|-------------|---------|-------------|------------|-------------|---------|-------------|------------|--------------------------|-------|------|
| | | | | | | | | Data - | Total | | | | | | | | |
| NS/EW Streets: | | Green Va | alley Rd | | - | Green Val | ley Rd | | | Deer Vall | ley Rd | | | Deer Val | ley Rd | | |
| NOON | | NORTH | | | | SOUTHE | | | | EASTBO | | | | WESTE | | | - |
| NOON | 1 NL | 1 NT | 1 NR | 0 NU | 1 SL | 1 ST | SR SR | 0 SU | O EL | ET ET | 0 ER | 0 EU | 0 WL | WT | 0 | WU | TOT |
| 11:00 AM | 3 | 88 | 1 | 0 | 9 | 74 | 4 | 0 | 2 | 0 | 5 | 0 | 1 | 0 | WR 5 | 0 | 192 |
| 11:15 AM | 5 | 85 | 1 | o | 4 | 85 | 3 | o | 3 | 0 | 4 | ő | 2 | 1 | 6 | o | 199 |
| 11:30 AM | 2 | 83 | 3 | o | 6 | 91 | 5 | 0 | 5 | 0 | ż | 0 | 1 | Ô | 5 | o | 208 |
| 11:45 AM | 3 | 108 | 2 | 0 | 2 | 79 | 4 | Ō | 3 | 1 | 5 | ō | 4 | 1 | 8 | 0 | 22 |
| 12:00 PM | 4 | 81 | 4 | 0 | 4 | 86 | 6 | 0 | 1 | 0 | 2 | 0 | 2 | 0 | 13 | 0 | 20 |
| 12:15 PM | 7 | 96 | 0 | 2 | 6 | 93 | 3 | 0 | 7 | 0 | 5 | 0 | 1 | 0 | 6 | 0 | 22 |
| 12:30 PM | 2 | 98 | 0 | 0 | 5 | 89 | 5 | 0 | 4 | 0 | 2 | 0 | 2 | 0 | 10 | 0 | 21 |
| 12:45 PM | 4 | 108 | . 4 | 0 | 5 | 102 | 4 | 0 | 6 | 0 | 3 | 0 | 0 | 0 | 9 | 0 | 24 |
| 1:00 PM | 4 | 97 | 8 | 0 | 10 | 105 | 6 | 0 | 1 | 1 | 3 | 0 | 4 | 1 | 4 | 0 | 24 |
| 1:15 PM | 11 | 93 | 6 | 0 | 5 | 113 | 2 | 0 | 3 | 0 | 3 | 0 | 2 | 0 | 1 | 0 | 23 |
| 1:30 PM | 2 | 94 | 3 | 1 | 9 | 99 88 | 7 5 | 0 | 5 | 0 | 3 | 0 | 4 | 0 | 5 | 0 | 23 |
| 1:45 PM | 8 | 96 | 1 | U | 11 | 88 | 5 | U | 4 | 1 | 6 | 0 | 1 | 2 | 1 | 0 | 22 |
| | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL | WT | WR | WU | TOT |
| TOTAL VOLUMES : | 55 | 1127 | 33 | 3 | 76 | 1104 | 54 | 0 | 44 | 3 | 48 | 0 | 24 | 5 | 73 | 0 | 264 |
| APPROACH %'s: | 4.52% | 92.53% | 2.71% | 0.25% | 6.16% | 89.47% | 4.38% | 0.00% | 46.32% | 3.16% | 50.53% | 0.00% | 23.53% | 4.90% | 71.57% | 0.00% | |
| PEAK HR : | | 2:45 PM - | | - | | *** | | | | | | | | | | | TOT |
| PEAK HR VOL : | 21 | 392 0.907 | 21 0.656 | 0.250 | 29 0.725 | 419 0.927 | 19 0.679 | 0.000 | 15 0.625 | 0.250 | 12 1.000 | 0,000 | 10 0.625 | 0.250 | 19 | 0 | 96 |
| PEAK HR FACTOR : | 0.477 | 0.907 | | 0.250 | 0.725 | 0.927 | | 0.000 | 0.025 | 0.77 | | 0.000 | 0.625 | 0.250 | 0.528 | 0.000 | 0.98 |
| | | 0.3. | 30 | | | 0.30 | | | | 0.77 | 0 | | | 0.0. | 13 | | |
| | · | NORTH | BOUND | | | SOUTHE | BOUND | | | EASTBO | OUND | | | WESTE | OUND | | |
| PM | 1 | 1 | 1 | 0 | 1 | 1 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | |
| | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL | WT | WR | WU | TOT |
| 2:00 PM | 4 | 93 | 1 | 0 | 7 | 126 | 0 | 0 | 6 | 1 | 8 | 0 | 2 | 0 | 5 | 0 | 25. |
| 2:15 PM | 9 | 86 | 7 | 1 | 7 | 124 | 9 | 0 | 3 | 0 | 3 | 0 | 1 | 0 | 4 | 0 | 25 |
| 2:30 PM | 3 | 90 | 7 | 0 | 2 5 | 105 | 2 | 0 | 3 | 0 | 1 | 0 | 5 | 0 | 7 | 0 | 21 |
| 2:45 PM 3:00 PM | 4 2 | 79 | 6 | 0 | 5 8 | 127 | 1 | 0 | 6 | 0 | 2 | 0 | 2 . | 0 | 6 | 0 | 24 |
| 3:15 PM | 5 | 99 | 2 | ő | 8 | 127 | 3 | 0 | 6 | 0 | 8 | 0 | 0 | ŏ | 4 | 0 | 26 |
| 3:30 PM | 3 | 82 | 1 | ŏ | 9 | 116 | 5 | 1 | 8 | 1 | 2 | o o | 4 | ŏ | 11 | o | 24 |
| 3:45 PM | 7 | 99 | î | ō | 10 | 128 | 3 | ô | 4 | Ô | 6 | 0 | 7 | ŏ | 7 | o | 27 |
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| 4:15 PM | 5 | 105 | 3 | o | 7 | 127 | 4 | 0 | 5 | 1 | 6 | 0 | 3 | 0 | 6 | 0 | 27 |
| 4:30 PM | 4 | 103 | 5 | 0 | 7 | 103 | 5 | 0 | 4 | 0 | 3 | 0 | 3 | 1 | 5 | 0 | 24 |
| 4:45 PM | 5 | 86 | . 4 | 0 | 5 | 134 | 9 | 0 | 3 | 0 | . 6 | 1 | 4 | 0 | 6 | 0 | 26 |
| 5:00 PM | 10 | 94 | 7 | 0 | 2 | 111 | 6 | 0 | 5 | 0 | 5 | 0 | 0 | 0 | 7 | 0 | 24 |
| 5:15 PM | 1 | 85 | 5 | 0 | 7 | 170 | 5 | 0 | 2 | 0 | 1 | 0 | 4 | 0 | 6 | 0 | 28 |
| 5:30 PM | 4 | 104 | 2 | 0 | 7 | 135 | 5 | 0 | 5 | 0 | 4 | 0 | 4 | 0 | 3 | 0 | 27 |
| 5:45 PM 6:00 PM | 3 | 99 | 3 | 0 | . 8 | 122 105 | 5 | 0 | | | 5 | 0 | 2 | 0 | 7 | 0 | 25 |
| 6:15 PM | 5 | 73 | 4 | Ô | 6 | 77 | 3 | 0 | 3 | o | 5 | 0 | 1 | 0 | 5 | 0 | 18 |
| 6:30 PM | 0 | 65 | 3 | ő | 5 | 97 | 2 | 0 | 3 | o | 2 | Ö | ó | 0 | 5 | 0 | 18 |
| 6:45 PM | 2 | 38 | 3 | o | 4 | 93 | 3 | o | 2 | o | 5 | ő | ő | 1 | 2 | o | 15. |
| | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL | WT | WR | WU | TOT |
| TOTAL VOLUMES : | 87 | 1771 | 69 | 2 | 128 | 2401 | 77 | 1 | 78 | 3 | 81 | 1 | 48 | 2 | 103 | 0 | 48 |
| APPROACH %'s : | 4.51% | 91.81% | 3.58% | 0.10% | 4.91% | 92.10% | 2.95% | 0.04% | 47.85% | 1.84% | 49.69% | 0.61% | 31.37% | 1.31% | 67.32% | 0.00% | |
| | | | 04:45 PM | | 1 | | | | | | | | | | | | TO |
| PEAK HR : | | | | _ | | | | | | | | | | | | _ | |
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| | 2 3 2 | 10 | 1 1 2 | 8 | 0 0 0 | | 0 1 0 | 1 | 3 5 4 | 19 | | 116 116 89 106 | 410 | 165 139 132 | 565 | 3 4 6 8 | 24 | 6 9 14 8 | 40 | 290 268 241 239 | 1039 |
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| | 110 125 120 100 | 424 | 48 70 93 67 | 251 | 6 10 12 9 | 30 | 11 23 17 | 58 | 175 228 242 190 | 763 | | 41 32 35 46 | 161 | 70 56 77 55 | 297 | 1 2 2 | 13 | 6 4 2 6 | 17 | 118 94 116 107 | 488 |
| | 95 102 114 68 | 411 | 69 73 93 | 302 | 6 13 7 | 35 | 8 16 13 | 51 | 178 204 227 168 | 799 | | 23 27 25 29 | 121 | 52 64 81 48 | 252 | 1 2 3 | 6 | 2 2 1 0 | 11 | 78 95 110 77 | 390 |
| | 109 78 80 80 | 335 | 72 62 78 56 | 289 | 3 8 3 | 26 | 4 6 12 10 | 33 | 188 154 173 157 | 683 | | 24 15 9 | 77. | 37 41 40 20 | 166 | 2 6 0 | 8 | 5 1 1 | 7 | 68 63 50 35 | 258 |
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| | 56 61 74 86 104 | 231 | 30 20 40 44 48 | 103_ | 1 5 3 3 | 6 | 7 4 8 9 | 17 | 89 123 141 164 | 357 | | 67 54 58 55 | 266 | 106 88 85 74 81 | 405 | 4 5 15 8 8 | 31 | 7 7 6 7 6 | 34 | 183 167 160 147 150 | 736 |
| | 105 129 114 114 126 | 424 | 65 105 68 68 93 | 262 | 4 15 5 7 8 | 25 | 23 16 7 8 17 | 56 | 197 265 194 197 244 | 767 | | 33 42 35 | 194 | 75 66 73 83 58 | 296 | 5 5 4 7 9 | 26 | 3 10 5 7 1 | 26 | 124 121 115 139 103 | 542 |
| | 100 101 78 80 | 454 | 107 86 60 79 | 336 | 7 7 6 | 23 | 8 18 9 | 40 | 218 202 163 174 | 853 | | 31 34 21 20 | 141 | 58 60 43 39 | 272 | 3 2 3 0 | 23 | 4 7 2 1 | 17 | 96 103 69 60 | 453 |
| | 97 78 87 89 71 | 356 | 63 66 62 85 78 | 288 | 11 6 7 3 7 | 23 | 6 8 3 5 | 46 | 182 156 164 180 161 | 721 661 | | 18 19 11 7 8 | 93 | 33 31 31 22 8 | 175 92 | 4 1 1 0 | 9 | 3 0 1 3 | 7 | 57 57 43 31 19 | 150 |
| | 87 77 74 84 | 322 | 71 78 79 81 | 309 | 7 7 9 8 | 31 | 10 12 9 15 | 46 | 175 174 171 188 | 708 | | 7 8 7 6 | 28 | 18 11 10 10 | 49 | 0 1 1 0 | 2 | 0 0 0 1 | 1 | 25 20 18 17 | 80 |
| 0 | | 2288 | | 1692 | | 141 | | 249 | | | 0 | | 3031 | | 4269 | | 290 | | 333 | | |
| Р | | 52.4% | | 38.7% | | 3.2% | | 5.7% | | | Р | | 38.3% | | 53.9% | | 3.7% | | 4.2% | | |
| | | | 0 | | | 19 | N | | | | | | | | 116 | | 10 1 | | | | |
| M P M P P | | 07:45 483 0.936 878 | | 11:45 370 0.916 598 | | 07:45 35 0.583 48 | | 07:00 56 0.609 96 | | | PM P PM P P | | 13:45 413 0.930 735 | | 15:30 569 0.906 1089 | | 15:30 44 0.733 58 | | 13:45 57 0.792 58 | | |
| P P | | 07:45 483 0.936 | | 08:00 336 0.785 | | 07:45 35 0.583 | | 07:00 56 0.609 | | | P | | 16:15 388 0.924 | | 17:00 561 0.893 | | 16:00 36 0.750 | | 16:45 40 0.769 | | |

T ursday 5/13/2021 ☐ Dorado Hills CA21_070069_001

| | | | 0 | | | - | N | | | | | | | | | | | | | | |
|-----------------|--------------------------|------------------------------|----------------------------|------------------------------|-------------------|----------------------------|---------------------|----------------------------|--------------------------|-----|-------------------|-----------------------------|------------------------------|--------------------------|-------------------------------|-------------------------|----------------------------|---------------------|----------------------------|--------------------------|------|
| MP | N | | | | | | | | 0 | | PM P | N | | | | | | | | O | |
| | 7 1 2 2 | 12 | 5 7 4 8 | 24 | 0 0 0 | | 0 0 0 0 | | 12 8 6 10 3 | 36 | | 94 101 92 66 84 | 353 | 81 107 96 99 | 383 | 4 11 7 10 6 | 32 | 9 11 13 5 | 38 | 188 230 208 180 | 806 |
| | 5 1 4 | 12 | 4 0 2 | 7 | 0 1 0 | 1 | 2 0 0 | 2 | 11 2 6 | 22 | | 83 90 78 | 335 | 97 101 97 | 392 | 9 2 9 | 26 | 9 6 3 | 22 | 198 199 187 | 775 |
| | 1 1 4 0 | 6 | 3 2 0 5 | 10 | 0 0 0 1 | 1 | 0 0 0 | | 4 3 4 6 | 17 | | 117 88 99 126 | 430 | 96 120 135 108 | 459 | 9 7 7 9 | 32 | 8 5 10 12 | 35 | 230 220 251 255 | 956 |
| | 2 1 4 3 | 10 | 2 6 4 1 | 13 | 0 0 0 | | 0 0 0 1 | 1 | 4 7 8 5 | 24 | | 92 107 140 112 | 451 | 157 149 153 155 | 614 | 8 12 8 13 | 41 | 9 5 8 12 | 34 | 266 273 309 292 | 1140 |
| | 6 5 6 13 | 30 | 2 2 2 3 | 9 | 0 0 0 | | 0 2 3 3 | 8 | 8 9 11 19 | 47 | | 94 112 112 92 | 410 | 127 122 149 196 | 594 | 16 10 3 7 | 36 | 14 12 9 10 | 45 | 251 256 273 305 | 1085 |
| | 19 19 40 34 | | 5 5 9 | | 0 0 0 | | 1 3 7 5 | | 25 27 56 55 | 163 | | 109 103 92 101 | 405 | 142 147 152 126 | 567 | 7 10 11 7 | 35 | 14 16 11 9 | 50 | 272 276 266 243 | 1057 |
| | 38 61 68 69 | 236 | 16 11 31 27 25 | 35 94 | 0 1 1 4 | 6 | 3 6 2 9 | 16 | 52 99 98 107 | 356 | | 67 80 78 65 | 290 | 99 110 101 80 | 390 | 7 11 10 3 | 31 | 8 4 3 6 | 21 | 181 205 192 154 | 732 |
| | 93 110 121 138 | 462 | 50 51 50 112 | 263 | 3 4 5 10 | 22 | 6 11 21 19 | 57 | 152 176 197 279 | 804 | | 62 61 59 41 | 223 | 77 99 62 72 | 310 | 8 9 4 12 | 33 | 7 10 5 2 | 24 | 154 179 130 127 | 590 |
| | 101 115 136 117 | 469 | 63 72 88 103 | 326 | 6 7 2 6 | 21 | 8 9 13 9 | 39 | 178 203 239 235 | 855 | | 41 36 43 36 | 156 | 53 71 70 70 | 264 | 11 2 4 4 | 21 | 3 4 3 2 | 12 | 108 113 120 112 | 453 |
| | 81 102 106 70 | 359 | 77 70 62 72 | 281 | 6 5 5 | 22 | 10 8 9 10 | 37 | 174 186 182 157 | 699 | | 24 25 20 15 | 84 | 52 51 40 48 | 191 | 6 5 1 5 | 17 | 2 3 4 | 12 | 84 84 64 72 | 304 |
| | 80 87 82 79 | 328 | 69 63 74 84 | 290 | 6 5 1 6 | 18 | 9 8 12 12 | 41 | 164 163 169 181 | 677 | | 14 13 12 8 | 47 | 38 29 25 21 | 113 | 0 1 2 1 | 4 | 0 0 1 1 | 2 | 52 43 40 31 | 166 |
| | 90 105 92 83 | 370 | 62 68 95 82 | 307 | 1 6 8 1 | 16 | 8 9 10 | 35 | 161 187 204 176 | 728 | | 8 6 5 4 | 23 | 16 12 9 9 | 46 | 1 0 1 | 3 | 0 2 1 1 | 4 | 25 20 16 15 | 76 |
| 0 | | 2406 | | 1659 | | 107 | | 256 | | | 0 | | 3207 | | 4323 | | 311 | | 299 | | |
| Р | | 54.3% | | 37.5% | | 2.4% | | 5.8% | | | Р | | 39.4% | | 53.1% | | 3.8% | | 3.7% | ų) | |
| | | | 0 | | | , d | N | | | | | | | | | | | | | | |
| M P M P P | | 07:45 490 0.888 | | 11:45 366 0.855 | | 07:30 28 0.700 | V | 07:15 59 0.702 | | | PM P PM P P | | 14:45 465 0.830 | - 1/1 | 16:45 637 0.813 | | 15:15 49 0.766 | | 16:45 51 0.797 | | |
| P P | | 931 07:45 490 0.888 | | 589 07:45 335 0.748 | | 43 07:30 28 0.700 | | 96 07:15 59 0.702 | | | P | | 815 16:15 425 0.949 | | 1161 16:45 637 0.813 | | 71 16:00 36 0.563 | | 95 16:45 51 0.797 | | |

P N

O M Green Valley Rd & Deer Valley Rd

riday 5/14/2021 ☐ Dorado Hills P CA21_070069_001

| | | | | | _ | | 2.1 | | | | | | | | | _ | | | | | |
|----|-----------|--------------|----------|--------------|----|-------------|---------|-------------|------------|-----|------|----------|--------------|------------|--------------|----------|-------------|----------|-------------|------------|-----|
| | | | 0 | | | | N | | | | | | - | | | | | | - | | - |
| MP | N | | | | | | | | 0 | | PMP | N | | | | | | | | C | |
| | 6 | | 7 | | 0 | | 0 | | 13 | | | 70 | | 99 | | 3 | | 14 | | 186 | |
| | 5 | | 12 8 | | 0 | | 0 | | 17 | | | 86 | | 89 94 | | 9 | | 13 14 | | 192 205 | |
| | 5 | 22 | 6 | 33 | 2 | 2 | 2 | 2 | 15 | 59 | | 84 | | 117 | 399 | 10 | 26 | 13 | 54 | 224 | 80 |
| | 0 | | 4 | | 0 | | 0 | | 4 | | | 85 | | 113 | | 5 | | 4 | | 207 | |
| | 6 | | 6 | | 0 | | 0 | | 12 7 | | | 88 | | 143 117 | | 6 | | 8 10 | | 245 213 | |
| | 2 | 12 | 3 | 15 | 0 | | Ó | 1 | 5 | 28 | | 10 | | 131 | 504 | 10 | 23 | 12 | 34 | 259 | 92 |
| | 0 | | 3 | | 0 | | 0 | | 3 | | | 10: | | 103 | | 8 | | 11 | | 224 | |
| | 1 | | 3 | | 0 | | 0 | | 5 | | | 10 | | 128 132 | | 6 11 | | 9 | | 246 277 | |
| | 3 | 5 | 2 | 12 | 1 | 1 | Ö | | 6 | 18 | | 10 | | 124 | 487 | 11 | 36 | 12 | 42 | 250 | 99 |
| | 1 | | 2 | | 0 | | 0 | | 3 | | | 111 | | 123 | | 12 | | 12 | | 257 | |
| | 5 | | 4 | | 0 | | 0 | | 4 5 | | | 12 | | 170 171 | | 11 10 | | 13 7 | | 319 305 | |
| | 2 | 8 | 2 | 8 | 0 | | 1 | 1 | 5 | 17 | | 1.13 | | 124 | 588 | 6 | 39 | 9 | 41 | 257 | 113 |
| | 5 | | 2 | | 0 | | 1 | | 8 | | | 93 | | 166 | | 9 | | 10 | | 278 | |
| | 8 | | 1 | | 0 | | 2 | | 7 | | | 10 | | 157 125 | | 10 | | 5 | | 279 241 | |
| | 7 | 24 | 6 | 11 | 1 | 1 | 2 | 7 | 16 | 43 | | 10 | 3 406 | 151 | 599 | 7 | 34 | 5 | 25 | 266 | 106 |
| | 16 | | 2 | | 0 | | 2 | | 20 | | | 92 | | 122 | | 5 | | 8 | | 227 | |
| | 26 39 | | 9 | | 1 | | 2 | | 33 57 | | | 10 | | 146 146 | | 6 8 | | 7 11 | | 266 264 | |
| | 48 | 129 | 9 | 24 | 1 | 4 | 2 | 13 | 60 | 170 | | 90 | 388 | 125 | 539 | 4 | 23 | 14 | 40 | 233 | 99 |
| | 52 | | 14 | | 2 | | 2 | | 70 97 | | | 86 63 | | 110 | | 7 | | 15 | | 218 | |
| | 51 65 | | 37 25 | | 2 | | 7 2 | | 94 | | | 72 | | 107 86 | | 10 9 | | 4 11 | | 184 178 | |
| | 59 | 227 | 38 | 114 | 10 | 16 | 3 | 14 | 110 | 371 | | 61 | | 99 | 402 | 5 | 31 | 9 | 39 | 174 | 75 |
| | 86 95 | | 40 43 | | 9 | | 6 11 | | 134 158 | | | 54 37 | | 81 81 | | 4 | | 2 7 | | 141 | |
| | 126 | | 60 | | 10 | | 23 | | 219 | | | 46 | | 94 | | 2 | | 2 | | 144 | |
| | 157 | 464 | 90 | 233 | 8 | 29 | 15 | 55 | 270 | 781 | | 40 | | 76 | 332 | 3 | 11 | 8 | 19 | 127 | 53 |
| | 117 | | 72 51 | | 9 | | 12 7 | | 210 177 | | | 40 | | 66 67 | | 3 | | 6 | | 115 | |
| | 111 | | 90 | | 20 | | 9 | | 237 | | | 41 | | 56 | | 2 | | 3 | | 102 | |
| | 113 | 459 | 73 | 286 | 9 | 46 | 13 | 41 | 208 | 832 | | 35 | | 56 | 245 | 1 | 9 | 2 | 13 | 94 | 42 |
| | 90 | | 87 67 | | 10 | | 9 | | 196 169 | | | 24 | | 55 42 | | 3 | | 3 | | 102 73 | |
| | 117 | | 58 | | 7 | | 9 | | 191 | | | 22 | | 31 | | 2 | | 2 | | 57 | |
| | 89 | 384 | 78 | 290 | 6 | 29 | 1 | 27 | 174 | 730 | | 23 | | 47 | 175 | 4 | 14 | 0 | 7 | 74 | 30 |
| | 95 103 | | 75 73 | | 11 | | 9 | | 190 194 | | | 32 | | 56 46 | | 0 | | 2 | | 90 62 | |
| | 104 | | 73 | | 7 | | 8 | | 192 | | | 15 | | 43 | | 1 | | 1 | | 60 | |
| | 84 | 386 | 82 63 | 303 | 10 | 34 | 5 8 | 30 | 177 | 753 | | 11 | | 23 | 168 | 1 | _1_ | 1 | 6 | 35 | 24 |
| | 93 | | 87 | | 7 | | 11 | | 199 | | | 11 | | 26 | | ò | | Ó | | 37 | |
| | 117 | | 90 | | 6 | | 13 | | 226 | | | 13 | | 18 | | 1 | | 0 | | 32 | |
| 0 | 80 | 384 2504 | 101 | 341 1670 | 9 | 32 194 | 13 | 45 236 | 203 | 802 | 0 | 4 | 39 3226 | 17 | 84 4522 | 0 | 249 | _1_ | 322 | 22 | 12 |
| Р | - | 54.4% | | 36.3% | | 4.2% | | 5.1% | | | Р | | 38.8% | , | 54.4% | | 3.0% | | 3.9% | | |
| | | | | 00.070 | | | N | | | | | | | | | | | | | 1100 | |
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| MP | | 07:30 | | 11:45 | | 08:15 | | 07:15 | | | PM P | | 15:00 | | 15:15 | | 14:30 | | 12:00 | | |
| MP | | 511 | | 383 0.948 | | 47 0.588 | | 61 0.663 | | | PM P | | 470 0.940 | | 631 0.923 | | 45 0.938 | | 54 0.964 | | |
| P | | 0.814 923 | | 519 | | 75 | | 96 | | | | | 794 | | 1138 | | 57 | | 65 | | |
| Р | | 07:30 | | 07:45 | | 08:00 | | 07:15 | | | Р | | 16:00 | | 16:00 | | 16:00 | | 17:00 | | |
| Р | | 511 | | 303 | | 46 | | 61 | | | | | 406 | | 599 | | 34 | | 40 | | |
| P | | 0.814 | | 0.842 | | 0.575 | | 0.663 | | | P | | 0.931 | | 0.902 | | 0.850 | | 0.714 | | |

a urday 5/15/2021 ☐ Dorado Hills P CA21_070069_001

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| MP | N | | | | | | | | 0 | TE | PM P | N | 5.5 | | 1 | | | | | 0 | |
| | 5 5 7 2 | 19 | 12 14 7 7 | 40 | 0 0 1 0 | 1 | 0 1 1 0 | 2 | 17 20 16 9 | 62 | | 96 87 96 95 | 374 | 97 146 110 102 | 455 | 6 6 6 | 24 | 7 9 15 14 | 45 | 206 248 227 217 | 89 |
| | 4 2 5 3 | 14 | 5 8 9 6 | 28 | 0 0 0 | | 0 0 0 | | 9 10 14 9 | 42 | | 96 108 103 95 | 402 | 121 110 96 123 | 450 | 2 4 6 5 | 17 | 5 15 5 15 | 40 | 224 237 210 238 | 90 |
| | 1 2 1 | 6 | 1 3 4 3 | 11 | 1 0 0 0 | 1 | 1 0 0 | 2 | 5 5 6 4 | 20 | | 85 96 100 100 | 381 | 102 119 100 122 | 443 | 6 7 7 9 | 29 | 4 7 7 16 | 34 | 197 229 214 247 | 88 |
| | 3 3 2 0 | 8 | 3 1 4 1 | 9 | 1 0 0 0 | 1 | 0 0 0 | | 7 4 6 1 | 18 | | 85 69 93 70 | 317 | 126 130 101 101 | 458 | 5 7 1 2 | 15 | 12 10 12 5 | 39 | 228 216 207 178 | 82 |
| | 3 5 6 5 | 19 | 3 1 3 4 | 11 | 0 0 0 | | 0 0 1 0 | 1 | 6 6 10 9 | 31 | | 93 94 79 72 | 338 | 103 102 142 108 | 455 | 7 5 11 9 | 32 | 7 7 12 10 | 36 | 210 208 244 199 | 86 |
| | 7 10 10 22 | 49 | 1 5 7 | 20 | 0 0 0 1 | 1 | 2 0 2 2 | 6 | 10 15 19 32 | 76 | | 81 93 67 67 | 308 | 93 83 79 70 | 325 | 8 7 6 8 | 29 | 7 14 15 15 | 51 | 189 197 167 160 | 71 |
| | 21 19 29 31 | 100 | 11 8 12 18 | 49 | 1 1 2 0 | 4 | 2 3 2 3 | 10 | 35 31 45 52 | 163 | | 74 62 59 53 | 248 | 100 88 73 79 | 340 | 2 11 4 | 19 | 12 9 9 7 | 37 | 188 161 152 143 | 6 |
| | 29 48 58 50 | 185 | 18 26 26 29 | 99 | 2 6 3 6 | 17 | 3 5 8 | 24 | 52 85 95 93 | 325 | | 52 50 39 42 | 183 | 83 67 58 50 | 258 | 4 6 5 7 | 22 | 5 8 3 5 | 21 | 144 131 105 104 | 4 |
| | 49 55 81 68 | 253 | 39 43 49 67 | 198 | 5 9 9 10 | 33 | 5 5 11 8 | 29 | 98 112 150 153 | 513 | | 41 34 34 30 | 139 | 55 62 60 50 | 227 | 4 4 6 1 | 15 | 4 6 5 6 | 21 | 104 106 105 87 | 41 |
| | 67 78 96 91 | 332 | 68 49 66 67 | 250 | 9 6 6 7 | 28 | 12 7 10 8 | 37 | 156 140 178 173 | 647 | | 29 31 29 30 | 119 | 57 40 53 63 | 213 | 3 1 2 3 | 9 | 2 1 3 1 | 7 | 91 73 87 97 | 3 |
| | 83 100 100 115 | 398 | 90 72 78 84 | 324 | 7 7 4 5 | 23 | 8 9 8 9 | 34 | 188 188 190 213 | 779 | | 32 20 27 18 | 97 | 29 25 31 | 129 | 0 2 1 0 | 3 | 6 4 1 1 | 12 | 82 55 54 50 | 2 |
| | 106 104 120 89 | 419 | 90 68 94 100 | 352 | 12 6 5 10 | 33 | 10 10 12 9 | 41 | 218 188 231 208 | 845 | | 10 14 9 6 | 39 | 24 32 31 11 | 98 | 2 2 2 2 | 8 | 1 3 3 2 | 9 | 37 51 45 21 | 1 |
| 0 | | 1802 | | 1391 | | 142 | | 186 | | | 0 | | 2945 | | 3851 | | 222 | | 352 | | |
| Р | | 51.2% | , b | 39.5% | | 4.0% | 1 | 5.3% | | | Р | | 40.0% | | 52.3% | | 3.0% | | 4.8% | | |
| | | | O | | | | N | | | | | | | | | | | | | | |
| M P | - (| 10:45 445 | Ų. | 11:45 453 | | 08:15 37 | | 10:45 41 | | | PM P PM P | | 12:45 402 | | 12:15 479 | | 16:30 35 | | 17:15 56 | | |
| P | | 0.927 438 08:00 | | 0.776 297 08:00 | | 0.925 50 08:00 | | 0.854 53 07:45 | | | Р | | 0.931 646 16:00 | | 780 16:00 | 1 | 0.795 61 16:30 | | 0.933 87 17:00 | | |
| Р | | 253 0.781 | | 198 0.739 | | 33 0.825 | | 29 0.659 | | | P | | 338 0.899 | | 455 0.801 | | 35 0.795 | | 51 0.850 | | |

P N

O M Green Valley Rd & Deer Valley Rd

unday 5/16/2021 El Dorado Hills P CA21_070069_001

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|----|----------|--------------|----------|--------------|---|-------------|----------|-------------|------------|-----|------|----------|--------------|-----------|--------------|--------|-------------|---------|-------------|------------|-----|
| | | | 0 | | | | N. | | | | | | | | | | | | | | |
| MP | N | | | | | | | | O | | PMP | N | | | | | | | | С | |
| | 7 | | 19 14 | | 1 | | 0 | | 27 26 | | | 88 82 | | 101 85 | | 10 | | 10 | | 209 | |
| | 12 | | 13 | | 0 | | 2 | | 21 | | | 86 | | 109 | | 3 5 | | 6 8 | | 176 208 | |
| | 7 | 32 | 4 | 50 | 0 | 1 | 2 | 4 | 13 | 87 | | 99 | 355 | 93 | 388 | 6 | 24 | 9 | 33 | 207 | 80 |
| | 2 | | 5 7 | | 0 | | 0 | | 7 | | | 83 85 | | 109 86 | | 7 | | 10 9 | | 209 189 | |
| | 3 | | 12 | | O | | 1 | 1 | 16 | | | 87 | | 96 | | 10 | | 6 | | 199 | |
| | 1 | 8 | 5 | 29 | 0 | | 0 | 2 | 6 | 39 | | 83 | 338 | 90 | 381 | 5 | 31 | 6 | 31 | 184 | 78 |
| | 0 4 | | 8 | | 0 | | 0 | | 8 | | | 78 78 | | 114 | | 2 | | 3 10 | | 197 186 | |
| | 0 | | 2 | | 0 | | 0 | | 2 | | | 81 | | 99 | | 6 | | 6 | | 192 | |
| | 4 | 8 | 2 | 19 | 0 | | 0 | | 6 | 27 | | 80 | 317 | 97 | 404 | 5 | 17 | 8 | 27 | 190 | 76 |
| | 5 | | 1 | | 0 | | 0 | | 2 | | | 59 71 | | 80 112 | | 2 | | 10 6 | | 153 191 | |
| | 0 | | 2 | | 0 | | 0 | | 2 | | | 85 | | 106 | | 3 | | 10 | | 204 | |
| | 1 | 10 | 1 | 5 | 0 | | 0 | 2 | 5 | 17 | | 68 85 | 283 | 109 | 407 | 5 | 14 | 12 5 | 38 | 194 | 74 |
| | 3 | | 3 | | 0 | | 1 | | 7 | | | 61 | | 109 | | 2 | | 9 | | 181 | |
| | 1 | | 1 | _ | 0 | | 1 | _ | 3 | | | 75 | | 77 | | 5 | | 10 | | 167 | |
| | 3 | 9 | 0 | 8 | 0 | | 3 | 5 | 10 | 22 | | 71 65 | 292 | 98 95 | 367 | 7 | 12 | 7 | 31 | 177 178 | 70 |
| | 9 | | 3 | | Ö | | ò | | 12 | | | 70 | | 88 | | 3 | | 6 | | 167 | |
| | 9 | | 5 | 40 | 0 | | 2 | 7 | 16 | 00 | | 64 | 0.17 | 86 | 200 | 6 | 00 | 7 | | 163 | 00 |
| | 18 | 39 | 5 | 13 | 0 | 1 | 3 | 7 | 28 | 60 | | 48 63 | 247 | 61 65 | 330 | 4 | 20 | 7 | 31 | 120 139 | 62 |
| | 12 | | 6 | | 1 | | 2 | | 21 | | | 51 | | 57 | | 3 | | 5 | | 116 | |
| | 11 | F4 | 10 | 00 | 1 | - | 2 | 40 | 24 | 00 | | 49 | 000 | 61 | 040 | 6 | 40 | 7 | 00 | 123 | 40 |
| | 15 | 51 | 6 | 32 | 0 | 5 | 3 | 10 | 32 | 98 | | 37 | 200 | 63 70 | 246 | 3 | 16 | 9 | 28 | 112 | 49 |
| | 28 | | 14 | | 2 | | 3 | | 47 | | | 42 | | 60 | | 8 | | 7 | | 117 | |
| | 42 35 | 133 | 21 34 | 75 | 3 | 9 | 3 5 | 15 | 69 78 | 232 | | 38 | 145 | 46 68 | 244 | 0 | 13 | 7 | 27 | 91 106 | 42 |
| | 29 | 133 | 29 | 73 | 5 | 3 | 3 | 13 | 66 | 202 | | 34 | 143 | 55 | 244 | 1 | 13 | 8 | 21 | 98 | 42 |
| | 43 | | 26 | | 4 | | 15 | | 88 | | | 50 | | 36 | | 2 | | 1 | | 89 | |
| | 43 63 | 178 | 34 29 | 118 | 7 | 20 | 3 11 | 32 | 87 107 | 348 | | 31 20 | 135 | 68 45 | 204 | 0 | 7 | 2 | 12 | 101 70 | 35 |
| - | 64 | 170 | 29 | 110 | 6 | | 8 | - OL | 107 | 010 | | 32 | 100 | 31 | 204 | 5 | - | 2 | 12 | 70 | - |
| | 65 | | 38 | | 7 | | 9 | | 119 | | | 23 | | 29 | | 1 | | 0 | | 53 | |
| | 90 | 308 | 58 51 | 176 | 8 | 25 | 10 5 | 32 | 166 149 | 541 | | 17 10 | 82 | 28 29 | 117 | 0 | 6 | 0 | 3 | 46 39 | 20 |
| | 73 | | 66 | | 5 | | 4 | | 148 | | | 12 | | 24 | | 1 | | 0 | | 37 | |
| | 86 88 | | 73 | | 9 | | 12 13 | | 180 | | | 12 | | 24 15 | | 0 | | 0 | | 38 26 | |
| | 90 | 337 | 75 75 | 289 | 2 | 24 | 17 | 46 | 184 | 696 | | 10 | 45 | 14 | 77 | 0 | 2 | 1 | 2 | 25 | 12 |
| | 80 | | 84 | | 2 | | 5 | | 171 | | | 10 | | 9 | | 0 | | 1 | | 20 | |
| | 90 | | 84 99 | | 4 | | 7 11 | | 185 206 | | | 7 4 | | 8 12 | | 0 | | 0 | | 15 16 | |
| | 89 | 348 | 100 | 367 | 8 | 21 | 8 | 31 | 205 | 767 | | 7 | 28 | 11 | 40 | Ö | | ő | 1 | 18 | 69 |
| 0 | | 1 101 | | 1181 | | 106 | | 186 | | | 0 | | 2467 | | 3205 | | 162 | | 264 | | |
| Р | | 49.8% | | 40.3% | | 3.6% | | 6.3% | | | Р | | 40.5% | | 52.6% | | 2.7% | | 4.3% | | |
| | | | 0 | | | | N | | | | | | | | | | | | | | |
| MP | | 11;15 | | 11:45 | | 11:15 | | 10:15 | | | PM P | | 12:00 | | 15:15 | | 12:45 | | 15:00 | | 227 |
| MP | | 356 | | 395 | | 29 | | 47 | | | PM P | | 355 | | 410 | | 32 | | 38 | | |
| Р | | 0.989 | | 0.906 | | 0.725 | | 0.691 | | | Р | | 0.896 | | 0.915 | | 0.800 | | 0.792 | | |
| Р | | 311 08:00 | | 193 07:45 | | 29 07:45 | | 47 08:00 | | | Р | | 539 16:00 | | 697 16:15 | | 32 17:00 | | 62 16:15 | | |
| P | | 178 | | 123 | | 20 | | 32 | | | - | | 292 | | 379 | | 20 | | 37 | | |
| P | | 0.706 | | 0.904 | | 0.714 | | 0.533 | | | Р | | 0.859 | | 0.869 | | 0.714 | | 0.841 | | |

onday 5/17/2021 ☐ Dorado Hils CA21_070069_001

| 100 | | | o | | | | N | | | | | | | | 1 | | | | | | |
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| MP | N | | | | | | | | 0 | | PM P | N | | | | | | | | 0 | 9 |
| | 3 3 2 3 | 11 | 5 6 8 3 | 22 | 0 0 0 | | 0 0 1 0 | 1 | 8 9 11 6 | 34 | | 66 69 85 78 | 298 | 100 78 86 99 | 363 | 8 2 6 9 | 25 | 9 7 10 3 | 29 | 183 156 187 189 | 715 |
| | 5 2 2 3 | 12 | 4 8 2 1 | 15 | 0 0 0 | | 0 0 0 | | 9 10 4 | 27 | | 91 74 67 86 | 318 | 96 94 91 | 369 | 9 4 10 12 | 35 | 7 9 15 10 | 41 | 195 183 186 199 | 763 |
| | 1 3 4 5 | 13 | 2 3 0 2 | 7 | 0 0 0 | | 0 2 0 0 | 2 | 3 8 4 7 | 22 | | 78 108 97 95 86 | 378 | 120 108 123 131 | 482 | 8 7 9 10 | 34 | 10 12 5 4 | 31 | 216 235 234 240 233 | 925 |
| | 1 1 3 6 | 5 | 4 3 0 2 | 9 | 0 0 0 | | 0 1 0 | 1 | 5 5 3 8 | 15 | | 95 109 71 99 | 361 | 136 134 148 131 | 553 | 6 13 6 | 31 | 11 15 9 | 41 | 248 271 234 244 | 986 |
| | 6 7 12 | 31 | 2 3 4 | 11 | 0 0 0 | | 1 2 3 | 6 | 9 12 19 | 48 | | 99 83 97 89 | 378 | 125 108 130 129 | 494 | 6 14 11 10 | 37 | 10 8 8 7 | 34 | 240 213 246 235 | 943 |
| | 31 28 37 45 | 106 | 6 6 9 | 25 | 0 0 1 | 1 | 4 2 6 5 | 13 | 41 36 53 67 | 145 | | 76 77 65 49 | 307 | 117 133 123 98 | 502 | 4 9 8 3 | 31 | 5 14 4 5 | 30 | 202 233 200 155 | 870 |
| | 46 66 60 77 | 217 | 28 25 18 37 | 87 | 1 0 0 | 2 | 3 2 13 | 23 | 78 93 91 127 | 329 | | 74 39 56 56 | 218 | 99 72 78 58 | 347 | 13 2 9 5 | 27 | 6 6 8 | 25 | 192 119 151 123 | 617 |
| | 102 117 121 103 | 417 | 49 71 90 60 | 247 | 2 6 10 11 | 19 | 7 25 14 16 | 58 | 160 219 235 190 | 741 | | 54 36 28 37 | 174 | 55 52 64 69 | 229 | 5 6 4 | 21 | 5 7 4 | 20 | 119 100 102 114 | 444 |
| | 108 134 113 78 | 458 | 63 73 64 80 | 260 | 8 7 3 5 | 29 | 5 12 5 | 38 | 184 226 185 173 | 785 | | 33 33 25 29 | 128 | 64 53 46 44 | 232 | 3 5 5 | 17 | 3 4 3 | 12 | 101 94 80 79 | 389 |
| | 79 107 79 79 | 343 | 53 62 57 79 | 252 | 6 8 6 | 25 | 11 7 12 6 | 40 | 149 184 154 170 | 660 | | 10 10 14 | 73 | 23 26 23 | 138 | 2 1 0 | 8 | 1 4 0 | 8 | 72 39 37 38 | 227 |
| | 91 87 101 88 | 358 | 61 65 81 82 | 286 | 2 5 7 | 20 | 13 12 6 2 | 37 | 167 169 195 175 | 701 | | 6 7 7 | 34 | 20 14 18 21 | 75 | 0 1 0 1 | 1 | 0 3 | 5 | 27 22 28 29 | 115 |
| | 77 83 94 | 342 | 94 99 78 | 353 | 6 14 5 | 28 | 17 7 7 | 33 | 194 203 184 | 756 | | 11 4 3 | 25 | 8 12 10 | 51 | 0 0 | 2 | 0 0 | 070 | 20 16 13 | 78 |
| P | | 2313 54.3% | | 1574 36.9% | | 2.9% | 4 | 252 5.9% | | | O P | - | 2692 38.1% | | 3835 54.2% | | 269 | | 276 3.9% | | |
| | | | 0 | 00.378 | | 2.376 | N | J.3 /6 | | | | | 55.176 | | O1.276 | | 0.070 | | 0.076 | | |
| MP MP P | | 07:45 466 0.869 | | 11:15 371 0.928 | | 07:45 36 0.818 | | 07:15 62 0.620 | | | PM P PM P P | | 14:15 386 0.894 | | 15:00 553 0.934 | | 16:15 41 0.732 | | 13:30 47 0.783 | | |
| P P | | 875 07:45 466 0.869 | | 507 07:45 286 0.794 | | 48 07:45 36 0.818 | | 96 07:15 62 0.620 | | | P r | | 685 16:00 378 0.955 | | 996 16:45 509 0.957 | | 68 16:15 41 0.732 | | 64 16:00 34 0.850 | | |



Attachment B Warrant Analysis Worksheet

TRAFFIC SIGNAL VOLUME WARRANT ANALYSIS (2012 MUTCD)

| MAJOR STREET: | Green Valley Road | NB | SB | # OF APPROACH LANES: | 1 |
|---------------|----------------------|----|----|----------------------|---|
| MINOR STREET: | Deer Valley Road | EB | WB | # OF APPROACH LANES: | 1 |
| CITY, STATE: | El Dorado County, CA | | | | |
| COMMENTS: | Existing Conditions | | | | |
| | | | | | |

ISOLATED COMMUNITY WITH POPULATION LESS THAN 10,000 (Y OR N): 85TH PERCENTILE SPEED GREATER THAN 40 MPH ON MAJOR STREET (Y OR N):

| Г | N. | |
|---|----|---|
| Г | Y | _ |

| | | | MAJOR ST | MINOR ST | Ped Count | WARRANT | 1 - Conditi | on A Part 1 | WARRANT | 1 - Conditi | on B. Part 1 | WARRANT | 1 - Condition | on A, Part 2 | WARRANT | 1 - Condition | on B, Part 2 | WARRANT 2 | WARRANT 3 | WARF | ANT 4 |
|-----------|-------|----------|--------------------|----------------------|-----------|-----------|-------------|-------------|-----------|-------------|--------------|-----------|---------------|--------------|---------------|----------------|--------------|---------------|-----------|---------------------------|---------------------------|
| | | | TWO-WAY TRAFFIC | TRAFFIC HEAVY LEG | CROSSING | MAIN LINE | SIDE | | MAIN LINE | SIDE | | MAIN LINE | SIDE | BOTH MET | MAINLINE | SIDE STREET | BOTH MET | Four-Hour | Peak Hour | Ped Volume (Four-hour) | Ped Volume (Peak-hour) |
| THRESHOLD | VALUE | s | | - | | 350 | 105 | | 525 | 53 | | 280 | 84 | | 420 | 42 | | 60 | 75 | 100 | 190 |
| 02:00 AM | TO | 03:00 AM | | | | | | | | | | | | | | | | | | | |
| 03:00 AM | TO | 04:00 AM | | | | | | | | | | | | | | | | | | | |
| 04:00 AM | TO | 05:00 AM | | | | | | | | | | | | | | | | | | | |
| 05:00 AM | TO | 06:00 AM | | | | | | | | | | | | | | - | | | | | ***** |
| 06:00 AM | TO | 07:00 AM | | | | | | | | | | | | | | | | | | | |
| 07:00 AM | ТО | 08:00 AM | | | | | | | | - | | | | | | | | | | | |
| 08:00 AM | TO | 09:00 AM | | | | | | | | | | | | | | | | | | | |
| 09:00 AM | TO | 10:00 AM | | | | | | | | | | - | | | | | | | | | |
| 10:00 AM | TO | 11:00 AM | | | | | | | | | | | | | | | | | | | |
| 11:00 AM | TO | 12:00 PM | 750 | 35 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 12:00 PM | TO | 01:00 PM | 818 | 43 | 0 | Y | | | Y | | | Y | | | Y | Υ | Y | | | | |
| 01:00 PM | TO | 02:00 PM | 884 | 30 | 0 | Y | | | Y | | | Y | | | Y | | | | - | | |
| 02:00 PM | TO | 03:00 PM | 900 | 31 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 03:00 PM | TO | 04:00 PM | 934 | 40 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 04:00 PM | TO | 05:00 PM | 1,012 | 36 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 05:00 PM | TO | 06:00 PM | 998 | 33 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 06:00 PM | TO | 07:00 PM | 692 | 27 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 07:00 PM | TO | 08:00 PM | | | | | | | | | | | | | | | | | | | |
| 08:00 PM | TO | 09:00 PM | | | | | | | | | | | | | | | | | | | |
| 09:00 PM | TO | 10:00 PM | | | | | | | | | | | | | | | | | | | |
| | | | 6,988 | 275 | | 8 | 0 | 0 | 8 | 0 | 0 | В | 0 | 0 | 8 | 1 | 1 | 0 | 0 | 0 | 0 |
| | | | | | | 8 H | OURS NEE | DED | 8 H | OURS NEE | DED | | 8 HOURS | S NEEDED fo | or both Condi | tion A & B | | 4 HRS NEEDED | | | 1 HR NEED |
| | | | | | | N | OT SATISFI | ED | N | OT SATISF | ED | | | NOT SA | TISFIED | | | NOT SATISFIED | NOT | NOT SATISFIED | SATISFIE |



Memorandum

To:

Paul Lopez, Blue Mountain Inc.

From:

Stephen Dillon, E.I.T.

Robert Paderna, P.E., RSP₁

Re:

Summer Brook - Oakhaven

Green Valley Road/Deer Valley Road Intersection Summary

Date:

November 21, 2022

RECEIVED

MAR 2 0 2023

EL DORADO COUNTY
PLANNING AND BUILDING DEPARTMENT

The purpose of this memorandum is to document current intersection operations at Green Valley Road and Deer Valley Road (the "study intersection"), compare and contrast intersection volumes collected and analyzed between study years, and summarize previous analysis findings at the study intersection completed as part of the Summer Brook residential development project. Kimley-Horn previously conducted an analysis of the study intersection as part of the Traffic Impact Analysis (TIA) prepared for the Summer Brook development in February 2007, and have performed subsequent assessments of the study intersection in 2019, 2021, and 2022. This supplemental analysis is intended to inform and add context to recent conversations between Blue Mountain Inc., (the "Client") and El Dorado County ("County") regarding current operating conditions of the study intersection and the project's conditions of approval (COA).

Analysis Background

The project proposes to construct a total of 29 single-family (detached) homes. Access to the site will be provided via two full-access driveways along Green Valley Road, east of the study intersection. As part of the development review process, a traffic impact analysis (TIA) for the proposed project was completed by Kimley-Horn in February 2007¹. The 2007 TIA established 2006 as the Existing year and 2025 as the Cumulative condition year for evaluation and concluded the study intersection satisfied the California Manual on Uniform Traffic Control Devices (CA MUTCD) peak-hour signal warrant requirements during both the AM and PM peak-hours under both Cumulative 2025 No Project and Plus Project conditions.

As future year conditions established using El Dorado County's Travel Demand Model (TDM) have been updated since the 2007 TIA, the Client desired to reexamine the previously established signalization COA. Kimley-Horn compared CA MUTCD peak-hour warrant results from the 2007 TIA Cumulative 2025 scenario with CA MUTCD signal warrant results using traffic counts from January 2019 and May 2021. The updated CA MUTCD signal warrant evaluations (performed by Kimley-Horn and dated March 28, 2019² and June 4, 2021³ respectively) concluded that a traffic signal was not warranted for both No Project and Plus Project scenarios at the study intersection under the reestablished traffic conditions.

Similarly, intersection operations analysis from the 2007 TIA was revisited using the current roadway geometry and revised Near Term (2031) and Cumulative (2041) forecast traffic conditions developed from August 2021 traffic volume data and the updated El Dorado County TDM. The updated intersection evaluation (performed by Kimley-Horn and dated April 12, 2022⁴) concluded that the study intersection operated within the County's acceptable threshold for both Near Term (2031) No Project and Plus Project

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916 858 5800

¹ Ghori Property TIA, Kimley-Horn, February 14, 2007

² Summer Brook-Traffic Signal Warrant Evaluation - Green Valley Rd at Deer Valley Rd, Kimley-Horn, March 28, 2019

³ Summer Brook-Traffic Signal Warrant Evaluation - Green Valley Rd at Deer Valley Rd, Kimley-Horn, June 4, 2021

⁴ Green Valley Road/Deer Valley Road Intersection Operations Analysis, Kimley-Horn, April 12, 2022

Kimley » Horn

scenarios and beyond the County's acceptable threshold for both Cumulative (2041) No Project and Plus Project scenarios.

This updated summary memo by Kimley-Horn dated November 17, 2022, is presented to the County regarding consideration of traffic signalization of the study intersection and includes updated traffic operations analysis for both No Project and Plus Project conditions using October 2022 intersection turning movements, updated CA MUTCD peak-hour warrant results, and comparison of intersection operations, CA MUTCD warrant results, and volumes (turning movements and segment ADT) between the various analysis years described above.

Analysis Methodology

Level of Service Definitions

The level of service (LOS) of a facility is a qualitative measure used to describe operational conditions. LOS ranges from A, which represents minimal delay, to F, which represents heavy delay and a facility that is operating at or near its functional capacity. LOS for this study was determined using methods defined in the Highway Capacity Manual (HCM) 6th Edition.

Intersection Analysis

The HCM includes procedures for analyzing side-street stop controlled (SSSC) intersections. The SSSC procedure defines LOS as a function of average control delay for each minor street approach movement. Table 1 presents intersection LOS definitions as defined in the HCM.

Table 1 - Intersection Level of Service Criteria

| Level of | Un-Signalized |
|------------------|----------------------------------|
| Service (LOS) | Average Control Delay* (sec/veh) |
| Α | ≤ 10 |
| В | > 10 – 15 |
| С | > 15 – 25 |
| D | > 25 – 35 |
| Е | > 35 – 50 |
| F | > 50 |

Source: Highway Capacity Manual, 6th Edition * Applied to the worst lane/lane group(s) for SSSC

LOS for the study intersections was determined using the Synchro® traffic analysis software. Synchro is an interactive computer program that enables planners and engineers to: forecast the traffic impacts of new developments; conduct area-wide traffic forecasting studies; test different mitigation measures and compare different traffic scenarios. Synchro 11 utilizes HCM 6 methodology to analyze intersection delay and LOS. Level of service for the Intersection is evaluated against El Dorado County thresholds of LOS D for Rural Regions⁵.

Analysis Results

Synchro 11 analysis was conducted for the Intersection under Existing (2022) No Project and Plus Project conditions using present day intersection geometry. The results of the analysis, including results from prior Project analyses in 2007¹ and 2022⁴, are reported in Table 2.

Page 2 of 5

Summer Brook - Oakhaven Green Valley Road/Deer Valley Road Intersection Summary November 21, 2022

⁵ Transportation Impact Study Guidelines, El Dorado County Community Development Agency, November 2014

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The study intersection operates at a satisfactory level for the El Dorado County Rural Region under Existing (2022) No Project and Plus Project Existing (2022) scenarios. The project is shown to add nominal delay to the study intersection.

Under Cumulative (2041) conditions, the study intersection is anticipated to operate below the County's LOS D threshold under No Project and Plus Project conditions. However, the project does not result in an increase of 5 or more seconds of delay.

Table 2 - Intersection Levels of Service (Green Valley Rd/Deer Valley Rd)

| | Scenario | | LOS Threshold | Peak Hour | Delay (s) | LOS |
|-----------------------------------|--------------------------|----------------|------------------|--------------|---------------|------|
| | | No Project | | AM | 2.7(22.1 SB) | A(C) |
| ent | 5intin = (2022) | No Project | D | PM | 2.1(27.9 NB) | A(D) |
| Current Analysis | Existing (2022) | Dius Project | | AM | 2.8(22.6 SB) | A(C) |
| 0 4 | | Plus Project | | PM | 2.1(28.5 NB) | A(D) |
| | | No Project | | AM | 4.7(29.3 SB) | A(D) |
| eq | Near Term | No Project | D | PM | 2.8(31.3 NB) | A(D) |
| dat | (2031)* | Dive Drainet | 7 0 | AM | 4.7(30.1 SB) | A(D) |
| U ysis | | Plus Project | | PM | 2.8(32.1 NB) | A(D) |
| Previously Updated Analysis | | No Dunio et | | AM | 7.2(44.0 SB) | A(E) |
| Not. | Cumulative | No Project | - | PM | 4.3(45.0 NB) | A(E) |
| Pre | (2041)* | Di D i t | D | AM | 7.3(45.2 SB) | A(E) |
| | | Plus Project . | | PM | 4.4(46.3 NB) | A(E) |
| | | No Decide | | AM | N/A(635.7 SB) | F |
| | Cumulative | No Project | D | PM | N/A(954.9 SB) | F |
| | (2025) ⁺ | Dive Decicet | 7 | AM | N/A(656.9 SB) | F |
| S | | Plus Project | | PM | N/A(985.3 SB) | F |
| lysi | | No Dunio et | | AM | N/A(220.4 SB) | F |
| Ana | | No Project | D | PM | N/A(72.1 SB) | F |
| al / | EPAP (2011) ⁺ | Dive Desired | 7 0 | AM | N/A(231.1 SB) | F |
| Original Analysis | | Plus Project | | PM | N/A(75.3 SB) | F |
| ō | | No Duningt | | AM | N/A(25.7 SB) | D |
| | - 1 .1 (222-1+ | No Project | | PM | N/A(20.1 NB) | С |
| | Existing (2007) | DI D | D | AM | N/A(26.3 SB) | D |
| | | Plus Project | | PM | N/A(20.4 NB) | С |

Note: **Bold** represents deficient operations.

Side Street Stop Control (SSSC) reported as intersection delay followed by worst approach's delay

Signal Warrant Results

CA MUTCD peak-hour signal warrants were developed for the Intersection under Existing (2022) No Project and Plus Project conditions using present day intersection geometry. The updated signal warrant findings, including warrant results from prior Project analyses in 2007¹, 2019², and 2021³, are presented in Table 3.

^{*}Green Valley Road/Deer Valley Road Intersection Operations Analysis, Kimley-Horn, April 12, 2022

^{*}Ghori Property TIA, Kimley-Horn, February 14, 2007



The Intersection does not satisfy the peak-hour signal warrant requirements in either AM or PM periods under Existing (2022) No Project and Plus Project conditions per CA MUTCD standards accepted by El Dorado County.

Table 3 - Intersection CA MUTCD Peak-Hour Signal Warrant Results

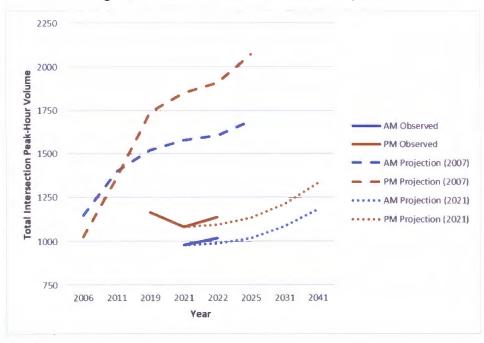
| | | | | Analysis : | Scenarios | | |
|---|---|---------------------------------|----------------------------------|-----------------------------------|---------------------------------|------------------------------|--------------------------------|
| # | Internation | Current Analysis | Previously Up | dated Analysis | | Original Analysis | |
| * | Intersection | Existing (2022) plus Project | Existing (2021) plus Project* | Existing (2019) plus Project** | Cumulative (2025) plus Project* | EPAP (2011) plus Project* | Existing (2007) plus Project* |
| 1 | Green Valley Road @ Deer Valley Road | No/No | No/No | No/No | Yes/Yes | Yes/Yes | Yes/No |

Results are presented in AM / PM format, locations where a signal is warranted during a peak hour are shaded

Volume Comparison

To better understand the broader context of the various analysis scenarios presented herein, Existing (2022) intersection turning movement counts collected as part of this effort were compared against intersection data utilized to perform HCM analysis and CA MUTCD signal warrant evaluations for the studies mentioned previously. The intersection turning movement volume comparison is presented in Figure 1. The comparison shows that volume growth projections based on the then-current El Dorado County TDM and used to establish future year conditions in the 2007 TIA proved more robust than the recently measured data. For example, the total measured AM peak-hour volumes from 2021 and 2022 were lower than the baseline established in 2007. The comparison also shows that the revised total intersection peak-hour volume projections from 2021 using the updated, current El Dorado County TDM forecast significantly lower traffic volume in future years than volumes established from the 2007 TIA, as depicted in the graph.

Figure 1 – Intersection Peak-Hour Volume Comparison



^{*}Summer Brook-Traffic Signal Warrant Evaluation - Green Valley Rd at Deer Valley Rd , Kimley-Horn, June 4, 2021

^{**}Summer Brook-Traffic Signal Warrant Evaluation - Green Valley Rd at Deer Valley Rd , Kimley-Horn, March 28, 2019

Ghori Property TIA, Kimley-Horn, February 14, 2007

Kimley » Horn

For additional context and to better understand travel behaviors after the effects of the 2020 COVID-19 pandemic, new daily segment counts (ADT) were collected across consecutive typical weekdays in 2022 at all four approaches to the study intersection. These 2022 volumes were compared to approach segment ADT data collected in 2019 (pre-pandemic) and 2021 (post-pandemic). This comparison is presented in Figure 2.

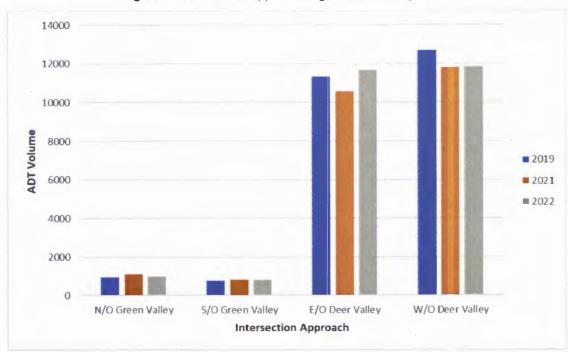


Figure 2 – Intersection Approach Segment ADT Comparison

The comparison shows that approach segment volumes in 2022 appear to have largely rebounded to 2019 pre-pandemic conditions, indicating that the underlying 2022 baseline data used in this analysis provides a stable platform from which to draw conclusions.

Attachments:

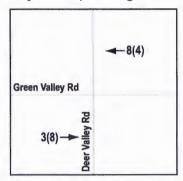
Exhibit 1 - Project Vicinity Diagram

Attachment 1 – Analysis Worksheets for Existing (2022) Conditions

Attachment 2 - Analysis Worksheets from Prior Studies

Attachment 3 – Signal Warrant Sheets for Existing (2022) Conditions

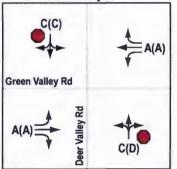
Project Trip Assignment

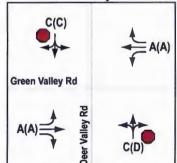


Existing (2022) Level of Service

No Project

Plus Project







LEGEND

Project Site
Study Intersection

A(A) AM(PM) Approach LOS

Intersection LOS threshold for Rural Regions is LOS D per El Dorado County Transportation Impact Study Guidelines





Attachment 1

Analysis Worksheets for Existing (2022) Conditions

Timing Plan: AM Peak Hour

| Para Caraca de C | | | | | | | | | | | | |
|--|--------|----------|--------|--------|----------|--------|----------|-------|-------|------------|--------|--------|
| Intersection | 0.7 | | | | | | The same | - | | | | |
| Int Delay, s/veh | 2.7 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 1 | ↑ | 7 | 7 | + | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 10 | 320 | 7 | 19 | 551 | 11 | 10 | 0 | 26 | 28 | 0 | 38 |
| Future Vol, veh/h | 10 | 320 | 7 | 19 | 551 | 11 | 10 | 0 | 26 | 28 | 0 | 38 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | | None | - | | None | | | None | | | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | - | - | - | - |
| Veh in Median Storage | e,# - | 0 | 10 -14 | | 0 | | - | 0 | | | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 89 | 89 | 89 | 93 | 93 | 93 | 70 | 70 | 70 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mymt Flow | 11 | 360 | 8 | 20 | 592 | 12 | 14 | 0 | 37 | 39 | 0 | 53 |
| | | | | | | | | | | | | |
| Major/Minor | Major1 | | | Major2 | | | Minor1 | 10.3 | | Minor2 | Esc | |
| Conflicting Flow All | 604 | 0 | 0 | 368 | 0 | 0 | 1047 | 1026 | 360 | 1037 | 1022 | 592 |
| Stage 1 | - | - | - | - | - | | 382 | 382 | - | 632 | 632 | - |
| Stage 2 | | | | | | - | 665 | 644 | - 35 | 405 | 390 | 7 |
| Critical Hdwy | 4.12 | | | 4.12 | | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | 7.12 | - | | 7.12 | | | 6.12 | 5.52 | 0.22 | 6.12 | 5.52 | 0.22 |
| Critical Hdwy Stg 2 | | | | | | | 6.12 | 5.52 | | 6.12 | 5.52 | |
| Follow-up Hdwy | 2.218 | _ | | 2.218 | | | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 974 | | · . | 1191 | | | 206 | 235 | 684 | 209 | 236 | 506 |
| Stage 1 | 314 | | | 1101 | | | 640 | 613 | 004 | 468 | 474 | 500 |
| Stage 2 | | | | | | | 449 | 468 | | TOTAL DOM: | 608 | |
| Platoon blocked, % | - | | | | _ | 120.00 | 110 | -100 | | ULL | 000 | |
| Mov Cap-1 Maneuver | 974 | | | 1191 | | | 181 | 228 | 684 | 194 | 229 | 506 |
| Mov Cap-1 Maneuver | 314 | _ | - | 1191 | | _ | 181 | 228 | - | 194 | 229 | 300 |
| Stage 1 | | - | | | | 75 | 633 | 606 | | 100 | 466 | |
| | | | | | | | 395 | 460 | | 582 | 601 | |
| Stage 2 | | 635 | - | - | <u> </u> | - | 333 | +00 | 19175 | 302 | 001 | - |
| With the second | | | | | | | *** | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | |
| HCM Control Delay, s | 0.3 | | | 0.3 | | | 15.8 | | | 22.1 | | |
| HCM LOS | | | | | | | С | | | С | | |
| | | - 3 | | | | | | 1 44 | with. | 2000 | | |
| Minor Lane/Major Mvr | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | hari i | 347.43 |
| Capacity (veh/h) | 17 | 386 | 974 | | | 1191 | - | | 301 | | 1 | |
| HCM Lane V/C Ratio | | | 0.012 | - | | 0.017 | | | 0.305 | | | |
| HCM Control Delay (s |) | 15.8 | 8.7 | | - | 8.1 | | | 22.1 | 100 | | |
| HCM Lane LOS | , | C | A | | | Α | | | С | | | |
| HCM 95th %tile Q(veh | 1) | 0.5 | 0 | | | 0.1 | - | | | | | |
| | 7 | 0.0 | | | | - | | | | | | |

| Intersection | | | | | | | | | | | | |
|------------------------|--------|----------|-------|------------|----------|-------|--------|------|-------|--------|-------|-------|
| Int Delay, s/veh | 2.1 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | ↑ | 7 | 7 | ^ | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 32 | 544 | 8 | 20 | 453 | 17 | 17 | 1 | 16 | 9 | 1 | 19 |
| Future Vol, veh/h | 32 | 544 | 8 | 20 | 453 | 17 | 17 | 1 | 16 | 9 | 1 | 19 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | - | None | 15- | | None | | | None | | | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | | - | | - | - | |
| Veh in Median Storag | e.# - | 0 | - | | 0 | | | 0 | | 153 | 0 | |
| Grade, % | | 0 | - | - | 0 | - | - | 0 | - | - | 0 | |
| Peak Hour Factor | 91 | 91 | 91 | 86 | 86 | 86 | 70 | 70 | 70 | 77 | 77 | 77 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 35 | 598 | 9 | 23 | 527 | 20 | 24 | 1 | 23 | 12 | 1 | 25 |
| | | | | | | | | | | | | |
| Major/Minor | Major1 | 200 | | Major2 | ng Sili | | Minor1 | | 10 | Minor2 | | 1 |
| Conflicting Flow All | 547 | 0 | 0 | 607 | 0 | 0 | 1264 | 1261 | 598 | 1258 | 1250 | 527 |
| Stage 1 | | | | The second | | 7-115 | 668 | 668 | - | 573 | 573 | |
| Stage 2 | | | | | | | 596 | 593 | | 685 | 677 | |
| Critical Hdwy | 4.12 | | | 4.12 | | 1 | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | | | | | | | 6.12 | 5.52 | - | 6.12 | 5.52 | |
| Critical Hdwy Stg 2 | | | | 1 | | - 0- | 6.12 | 5.52 | | 6.12 | 5.52 | |
| Follow-up Hdwy | 2.218 | | | 2.218 | | | 3.518 | | 3.318 | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 1022 | | 10 | 971 | | | 146 | 170 | 502 | 148 | 173 | 551 |
| Stage 1 | | - | | | | | 448 | 456 | - | 505 | 504 | - |
| Stage 2 | | | - | | | | 490 | 493 | | 438 | 452 | |
| Platoon blocked, % | | - | | | - | - | | | | | | |
| Mov Cap-1 Maneuver | 1022 | | | 971 | | 100 | 133 | 160 | 502 | 134 | 163 | 551 |
| Mov Cap-2 Maneuver | | | | | | | 133 | 160 | | 134 | 163 | |
| Stage 1 | | | | 1 | T. | | 433 | 440 | | 488 | 492 | |
| Stage 2 | | - | | | | - | 456 | 481 | - | 402 | 437 | |
| 4430 | | | | 4 | | | 949 | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | 1 |
| HCM Control Delay, s | 0.5 | | | 0.4 | | | 27.9 | | | 20.6 | | |
| HCM LOS | | | | | | | D | | | С | | |
| | | | | | | | | | | | | |
| Minor Lane/Major Mv | mt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | | E ES |
| Capacity (veh/h) | | 205 | | 1 - | | 971 | | 1 = | 269 | | | 113 |
| HCM Lane V/C Ratio | | | 0.034 | | | 0.024 | | | 0.14 | | | |
| HCM Control Delay (s | | 27.9 | 8.6 | 200 | -1-2 | 8.8 | - | | 20.6 | | 11 | |
| | , | D | A | | | A | | | C | | | |
| HCM Lane LOS | | U | A | - | _ | | | | | | | |

| Intersection | 0.0 | | | | | 5.0 | | | | | | |
|---|--------|----------|-------|--------|----------|-------|--------|------------|-------|--------|-------|------|
| Int Delay, s/veh | 2.8 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | † | 7 | 7 | ↑ | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 10 | 323 | 7 | 19 | 559 | 11 | 10 | 0 | 26 | 28 | 0 | 38 |
| Future Vol, veh/h | 10 | 323 | 7 | 19 | 559 | 11 | 10 | 0 | 26 | 28 | 0 | 38 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | - | None | - | 11. | None | | | None | | | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | | - | - | - |
| Veh in Median Storage | ,# - | 0 | | | 0 | | | 0 | | - | 0 | |
| Grade, % | _ | 0 | - | | 0 | | - | 0 | - | - | 0 | |
| Peak Hour Factor | 89 | 89 | 89 | 93 | 93 | 93 | 70 | 70 | 70 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 11 | 363 | 8 | 20 | 601 | 12 | 14 | 0 | 37 | 39 | 0 | 53 |
| | | | | | | | | | | | | |
| Major/Minor N | Major1 | | | Major2 | | | Minor1 | 23 | | Minor2 | | -335 |
| | 613 | 0 | 0 | 371 | 0 | 0 | 1059 | 1038 | 363 | 1049 | 1034 | 601 |
| Conflicting Flow All | 013 | | | 3/1 | - | - | 385 | 385 | 303 | 641 | 641 | |
| Stage 1 | - | | | | | - | 674 | 653 | | 408 | 393 | |
| Stage 2 | 4.12 | | - | 4.12 | | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Howy | | - | | 4.12 | - | • | 6.12 | 5.52 | 0.22 | 6.12 | 5.52 | 0.22 |
| Critical Hdwy Stg 1 Critical Hdwy Stg 2 | - | - | | | | | 6.12 | 5.52 | | 6.12 | 5.52 | |
| | 2.218 | | • | 2.218 | - | 100 | 3.518 | 4.018 | | 3.518 | 4.018 | |
| | 966 | | | 1188 | | _ | 202 | 231 | 682 | 205 | 232 | 500 |
| Pot Cap-1 Maneuver | | • | - | 1100 | - | - | 638 | | | 463 | 469 | 300 |
| Stage 1 | - | | | | | _ | 444 | 611 464 | | 620 | 606 | |
| Stage 2 | - | - | - | • | - | | 444 | 404 | - | 020 | 000 | - |
| Platoon blocked, % | 000 | - | | 4400 | - | | 477 | | 682 | 190 | 226 | 500 |
| Mov Cap-1 Maneuver | 966 | - | | 1188 | - | - | 177 | 225 | | | | 500 |
| Mov Cap-2 Maneuver | | | _ | - | - | - | 177 | 225 | | 190 | 226 | |
| Stage 1 | • | - | | | - | - | 631 | 604 | 95 | 458 | 461 | |
| Stage 2 | | | _ | _ | | | 390 | 456 | - | 580 | 599 | |
| | | | | | | | 2.40 | | | | | |
| Approach | EB | | | WB | | -0.4 | NB | | | SB | | |
| HCM Control Delay, s | 0.3 | | | 0.3 | | | 16 | | | 22.6 | | |
| HCM LOS | | | | | | | С | | | С | | |
| | | | | | | | | | | | 3.4 | |
| Minor Lane/Major Mvm | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | ă. | | |
| Capacity (veh/h) | | 380 | 966 | | | 1188 | | | 295 | | | |
| HCM Lane V/C Ratio | | | 0.012 | - | | 0.017 | - | | 0.311 | | | |
| HCM Control Delay (s) | | 16 | 8.8 | | - | 8.1 | - | | | | - 37 | |
| HCM Lane LOS | | С | Α | | | Α | | | С | | | |
| |) | 0.5 | 0 | | | 0.1 | | | 1.3 | - | - | |

| Intersection | | | | | | | | | | | | |
|---------------------------------------|--------|-----------|----------|--------|----------|----------|--------|-------|-----------|----------|-------|-----------|
| Int Delay, s/veh | 2.1 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | † | 7 | 7 | † | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 32 | 552 | 8 | 20 | 457 | 17 | 17 | 1 | 16 | 9 | 1 | 19 |
| Future Vol, veh/h | 32 | 552 | 8 | 20 | 457 | 17 | 17 | 1 | 16 | 9 | 1 | 19 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | | None | 1 | - 4 | None | | | None | - | | None |
| Storage Length | 415 | | 415 | 415 | - | 415 | | | | | - | |
| Veh in Median Storage | | 0 | | | 0 | - | | 0 | - | - | 0 | |
| Grade, % | | 0 | | | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 86 | 86 | 86 | 70 | 70 | 70 | 77 | 77 | 77 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 35 | 607 | 9 | 23 | 531 | 20 | 24 | 1 | 23 | 12 | 1 | 25 |
| | | | | | | | | | | | | |
| Major/Minor I | Major1 | | | Major2 | | | Minor1 | 900 | | Minor2 | | - |
| Conflicting Flow All | 551 | 0 | 0 | 616 | 0 | 0 | 1277 | 1274 | 607 | 1271 | 1263 | 531 |
| Stage 1 | 551 | - | - | - | - | | 677 | 677 | - | 577 | 577 | - |
| Stage 2 | | | | | | | 600 | 597 | | 694 | 686 | |
| Critical Hdwy | 4.12 | | | 4.12 | | - v . | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | 7.12 | | | | | | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | | | | | | 12.00 | 6.12 | 5.52 | | 6.12 | 5.52 | |
| Follow-up Hdwy | 2.218 | | | 2.218 | | | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 1019 | | 3.7. | 964 | | | 143 | 167 | 496 | 145 | 170 | 548 |
| Stage 1 | - | | | - | - | | 443 | 452 | - | 502 | 502 | - |
| Stage 2 | | | | | - | | 488 | 491 | | 433 | 448 | 1 |
| Platoon blocked, % | | | | | | | | | | | | |
| Mov Cap-1 Maneuver | 1019 | | | 964 | | | 130 | 157 | 496 | 131 | 160 | 548 |
| Mov Cap-2 Maneuver | - | | | | | | 130 | 157 | | 131 | 160 | - |
| Stage 1 | | | | | | - | 428 | 437 | | 485 | 490 | 1 10 |
| Stage 2 | - | | | - | | - | 454 | 479 | | 398 | 433 | |
| | | 1200 | | 100 | 97. | | | | | 100 | III. | |
| Approach | EB | (25) | | WB | 10 | | NB | 18.3 | | SB | | 0 100 |
| HCM Control Delay, s | 0.5 | | | 0.4 | | | 28.5 | | | 20.8 | | |
| HCM LOS | 0.0 | | | 3.1 | | | D | | | C | | |
| | | | | W. | | | | | | | | |
| Minor Lane/Major Mvn | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | 10 | |
| Capacity (veh/h) | | 201 | 1019 | - | | 964 | | | 265 | | HIM | |
| HCM Lane V/C Ratio | | | 0.035 | | | 0.024 | | | 0.142 | | | |
| TOTAL EURO TIO I LULIO | | | | | | | | | | Local at | 0.000 | III - NEW |
| HCM Control Delay (s) | | 28.5 | 8/ | | - | n n | - | | /11/8 | | | |
| HCM Control Delay (s) HCM Lane LOS | | 28.5 D | 8.7 A | 5 . | 0.00 | 8.8 A | | | 20.8 C | | 345.6 | all says |



| Atta | ch | m | er | ٦t | 2 |
|-------------|----|---|-----|----|---|
| π ιια | | | ~ I | 16 | _ |

Analysis Worksheets from Prior Studies

| Intersection | | | | | | | | | | | | |
|------------------------|--------|-----------|-------|--------|--------------|-------|--------|------|-----------|----------|------|-------|
| Int Delay, s/veh | 4.7 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | † | 7 | 7 | † | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 17 | 320 | 15 | 32 | 545 | 14 | 27 | 1 | 38 | 34 | 1 | 42 |
| Future Vol, veh/h | 17 | 320 | 15 | 32 | 545 | 14 | 27 | 1 | 38 | 34 | 1 | 42 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | | None | 11 /- | | None | | | None | 11 | | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | - | - | | - |
| Veh in Median Storage | e,# - | 0 | | 100 | 0 | | V 10 | 0 | | | 0 | |
| Grade, % | - | 0 | | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 19 | 352 | 16 | 37 | 626 | 16 | 39 | 1 | 55 | 47 | 1 | 58 |
| | | | | | | | | | | | | |
| Major/Minor | Major1 | | | Major2 | 1 | 100 | Minor1 | | 1 4 | Minor2 | | |
| Conflicting Flow All | 642 | 0 | 0 | 368 | 0 | 0 | 1128 | 1106 | 352 | 1126 | 1106 | 626 |
| Stage 1 | | | | | | | 390 | 390 | | 700 | 700 | 8 6 |
| Stage 2 | | | | | | | 738 | 716 | | 426 | 406 | |
| Critical Hdwy | 4.12 | | | 4.12 | 1 . | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | - | | | - | | | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | | all . | | | | | 6.12 | 5.52 | | 6.12 | 5.52 | - |
| Follow-up Hdwy | 2.218 | | | 2.218 | | | | | 3.318 | | | 3.318 |
| Pot Cap-1 Maneuver | 943 | - | | 1191 | 1712 | 1 | 181 | 210 | 692 | 182 | 210 | 484 |
| Stage 1 | - | | | - | | | 634 | 608 | | 430 | 441 | |
| Stage 2 | | | - | | | | 410 | 434 | | 606 | 598 | |
| Platoon blocked, % | | - | - | | | - | | | | | | |
| Mov Cap-1 Maneuver | 943 | | | 1191 | - | 40- | 152 | 200 | 692 | 160 | 200 | 484 |
| Mov Cap-2 Maneuver | | | | - | | - | 152 | 200 | | 160 | 200 | - |
| Stage 1 | | | | 1 | | | 621 | 596 | | 421 | 427 | |
| Stage 2 | | | - | | | | 348 | 421 | | 545 | 586 | |
| Part Town | | | | | 7.275.13 | | | | | | TE. | 10 |
| Approach | EB | | | WB | | | NB | | | SB | | |
| HCM Control Delay, s | 0.4 | | To Ta | 0.4 | THE STATE OF | | 24.6 | | | 29.3 | | 1150 |
| HCM LOS | | | | | | | С | | | D | | |
| Hydratia a | | | | 1 | 10 | | | | | 111 | | |
| Minor Lane/Major Mvr | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | | 1 |
| Capacity (veh/h) | | 278 | 943 | | | 1191 | | | 253 | | | |
| HCM Lane V/C Ratio | | 0.344 | 0.02 | | | 0.031 | | | 0.423 | | | - |
| HCM Control Delay (s | 1 | 24.6 | 8.9 | | TEN INC | 8.1 | | | 29.3 | | | 3 |
| HCM Lane LOS | 1 | 24,0 C | Α.9 | - | la III | Α. | | | 29.3 D | La Santa | | 11 - |
| HCM 95th %tile Q(veh | 2) | 1.5 | 0.1 | 1, - | | 0.1 | | - | 2 | | | |
| HOW SOM WHIE CALACI | 1) | 1.0 | 0.1 | 1100 | - | 0.1 | - 1 | | 2 | | | |

| Intersection | | | u. | | | | | | | | | |
|-------------------------------------|--------|----------|-----------|--------|-------|-------|-----------|------|---------|--------|--------|-------|
| Int Delay, s/veh | 2.8 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | * | 7 | 1 | * | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 34 | 626 | 14 | 33 | 397 | 28 | 18 | 2 | 25 | 11 | 1 | 23 |
| Future Vol, veh/h | 34 | 626 | 14 | 33 | 397 | 28 | 18 | 2 | 25 | 11 | 1 | 23 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | | None | | | None | | - | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | | - | - | | | - |
| Veh in Median Storag | | 0 | | | 0 | | 10. | 0 | - | | 0 | - |
| Grade, % | | 0 | - | - | 0 | - | - | 0 | _ | - | 0 | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mymt Flow | 37 | 688 | 15 | 38 | 456 | 32 | 26 | 3 | 36 | 15 | 1 | 32 |
| | | | | | | | | | | | | |
| Major/Minor | Major1 | | | Major2 | dia . | | Minor1 | V | | Minor2 | | |
| Conflicting Flow All | 488 | 0 | 0 | 703 | 0 | 0 | 1327 | 1326 | 688 | 1321 | 1309 | 456 |
| Stage 1 | - | | | - | | | 762 | 762 | - | 532 | 532 | - |
| Stage 2 | | | | - | | | 565 | 564 | | 789 | 777 | |
| Critical Hdwy | 4.12 | 198 | et et gel | 4.12 | | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | 7.12 | | | T. 16 | | _ | 6.12 | 5.52 | 0.22 | 6.12 | 5.52 | 0.22 |
| Critical Hdwy Stg 2 | | | | 1 | | | 6.12 | 5.52 | 57/10 | 6.12 | 5.52 | |
| Follow-up Hdwy | 2.218 | | | 2.218 | | | 3.518 | | | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 1075 | | | 895 | | 170 | 132 | 156 | 446 | 134 | 159 | 604 |
| Stage 1 | 1010 | | | 000 | | | 397 | 414 | - | 531 | 526 | - |
| Stage 2 | | | | 1 100 | | - | 510 | 508 | | 384 | 407 | |
| Platoon blocked, % | | | _ | | | | 010 | 500 | . 3 3 | 004 | 407 | |
| Mov Cap-1 Maneuver | 1075 | | | 895 | | | 117 | 144 | 446 | 114 | 147 | 604 |
| Mov Cap-1 Maneuver | | | _ | - 090 | | | 117 | 144 | - | 114 | 147 | 007 |
| Stage 1 | | | | | | | 384 | 400 | | 513 | 504 | |
| Stage 2 | | | | | | | 461 | 487 | - | 338 | 393 | |
| Staye 2 | | | | | | | 401 | 407 | 1000 | 330 | 223 | |
| Approach | EB | 0 5 5 | | WB | | | NB | E ST | | SB | 283 | |
| HCM Control Delay, s | | | | 0.7 | | | 31,3 | | | 23 | | |
| HCM LOS | 0.4 | | | 0.7 | | | 31.3 D | | | C | | |
| HOW LOS | | 16-6 | | | - 4 | | U | | | U | 1-1- | |
| Minor Lane/Major Mvi | mt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SRI n1 | - | 1-10-2 | - |
| | 116 | 201 | | LDI | LOIN | 895 | 11011 | WDIX | 248 | - | | |
| Capacity (veh/h) HCM Lane V/C Ratio | | | 0.035 | | • | 0.042 | | _ | 0.196 | 1 | | |
| | ., | | | | | | | | | 107 | | |
| HCM Control Delay (s |) | 31.3 | 8.5 | - | • | 9.2 | | - | 23 C | 1111 | | |
| HCM Lane LOS | L\ | D | A | | - | Α | | | | 100 | - | |
| HCM 95th %tile Q(vel | n) . | 1.3 | 0.1 | - | • | 0.1 | - | • | 0.7 | | 1 1 | |

| Intersection | | | | | | | | | | | | |
|-----------------------------|--------|-------|------|-----------|----------|-------|--------|-------|-------|--------|-------|-------|
| Int Delay, s/veh | 4.7 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | * | 7 | 7 | † | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 17 | 323 | 15 | 32 | 553 | 14 | 27 | 1 | 38 | 34 | 1 | 42 |
| Future Vol., veh/h | 17 | 323 | 15 | 32 | 553 | 14 | 27 | 1 | 38 | 34 | 1 | 42 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | | None | - | | None | | | None | | | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | - | | | - |
| Veh in Median Storage | e.# - | 0 | | | 0 | | | 0 | - | | 0 | - |
| Grade, % | - | 0 | - | | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mymt Flow | 19 | 355 | 16 | 37 | 636 | 16 | 39 | 1 | 55 | 47 | 1 | 58 |
| | | | | | | | | | | | | |
| Major/Minor | Major1 | | 13 | Major2 | TOT | 1 | Minor1 | | | Minor2 | NE T | |
| Conflicting Flow All | 652 | 0 | 0 | 371 | 0 | 0 | 1141 | 1119 | 355 | 1139 | 1119 | 636 |
| Stage 1 | - | | - | | ı. | - | 393 | 393 | - | 710 | 710 | |
| Stage 2 | | | | | | | 748 | 726 | | 429 | 409 | |
| Critical Hdwy | 4.12 | | 1 2 | 4.12 | - | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | - | | | - | | | 6.12 | 5.52 | - | 6.12 | 5.52 | |
| Critical Hdwy Stg 2 | | | | | - | | 6.12 | 5.52 | | 6.12 | 5.52 | 9 - |
| Follow-up Hdwy | 2.218 | | | 2.218 | | | 3.518 | 4.018 | 3.318 | | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 935 | | | 1188 | | 1 | 178 | 207 | 689 | 178 | 207 | 478 |
| Stage 1 | - | | | | | | 632 | 606 | | 424 | 437 | |
| Stage 2 | - | 1 | | | 7- | | 404 | 430 | - | 604 | 596 | |
| Platoon blocked, % | | | | | | - | | | | | | |
| Mov Cap-1 Maneuver | 935 | | | 1188 | | | 149 | 197 | 689 | 156 | 197 | 478 |
| Mov Cap-2 Maneuver | | - | | | | - | 149 | 197 | | 156 | 197 | |
| Stage 1 | - | - | | | | | 619 | 594 | | 416 | 423 | 1 . |
| Stage 2 | - | - | | | - | - | 343 | 417 | | 543 | 584 | - |
| | | | | 150 | | | | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | 1 23 |
| HCM Control Delay, s | 0.4 | | | 0.4 | | | 25.1 | | | 30.1 | 11. | |
| HCM LOS | | | | | | | D | | | D | | |
| Control of the | | | | 1 | | | | | | | | |
| Minor Lane/Major Mvr | nt I | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | 250 | i Est | |
| Capacity (veh/h) | 41.27 | 273 | 935 | | | 1188 | 77- | | 248 | | | |
| HCM Lane V/C Ratio | | 0.35 | 0.02 | | | 0.031 | | | 0.431 | | | |
| HCM Control Delay (s | () | 25.1 | 8.9 | | | 8.1 | | | 30.1 | | | 17 |
| HCM Lane LOS | , | D | A | | | A | | | D | | | |
| HCM 95th %tile Q(vel | h) | 1.5 | 0.1 | | - | 0.1 | | | 2 | | | 1111 |
| / / / / / / / / / / / / / / | 7 | 1.0 | 0.1 | 1 - 5 - 6 | | 0.1 | | | _ | | | |

| Intersection | | | | | | | | | | | | |
|------------------------|-------|----------|--------|-------|--------|-------|-------|--------|-------|-------|-------|-------|
| Int Delay, s/veh | 2.8 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | † | 7 | 7 | 1 | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 34 | 634 | 14 | 33 | 401 | 28 | 18 | 2 | 25 | 11 | 1 | 23 |
| Future Vol, veh/h | 34 | 634 | 14 | 33 | 401 | 28 | 18 | 2 | 25 | 11 | 1 | 23 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | | None | | | None | | | None | | | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | - | - | | - |
| Veh in Median Storage | e,# - | 0 | | - | 0 | - | | 0 | - | - | 0 | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 37 | 697 | 15 | 38 | 461 | 32 | 26 | 3 | 36 | 15 | 1 | 32 |
| | | | | | | | | | | | | |
| Major/Minor Major1 | | 2 | Major2 | | Minor1 | | | Minor2 | | | | |
| Conflicting Flow All | 493 | 0 | 0 | 712 | 0 | 0 | 1341 | 1340 | 697 | 1335 | 1323 | 461 |
| Stage 1 | 700 | - | | 112 | - | | 771 | 771 | - | 537 | 537 | 701 |
| Stage 2 | | | | - | - | - | 570 | 569 | | 798 | 786 | |
| Critical Hdwy | 4.12 | - | | 4.12 | | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | 7.12 | | | 7.12 | | | 6.12 | 5.52 | 0.22 | 6.12 | 5.52 | 0.22 |
| Critical Hdwy Stg 2 | | | | | | | 6.12 | 5.52 | | 6.12 | 5.52 | |
| Follow-up Hdwy | 2.218 | | | 2.218 | | | 3.518 | 4.018 | | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 1071 | | | 888 | | | 129 | 153 | 441 | 131 | 156 | 600 |
| Stage 1 | 10/1 | | | - | | | 393 | 410 | - | 528 | 523 | - |
| Stage 2 | | | | | | | 506 | 506 | | 380 | 403 | |
| Platoon blocked, % | | | _ | | _ | _ | 500 | 500 | | 000 | -100 | |
| Mov Cap-1 Maneuver | 1071 | | | 888 | | 311) | 114 | 141 | 441 | 111 | 144 | 600 |
| Mov Cap-1 Maneuver | - | - | | - | _ | _ | 114 | 141 | - | 111 | 144 | - |
| Stage 1 | | | | | | | 379 | 396 | | 510 | 501 | |
| Stage 2 | | - : | | | | | 457 | 484 | _ | 334 | 389 | - |
| Graye 2 | | | | | | | +01 | 704 | | JJ-4 | 303 | |
| Approach | EB | | | WB | | | NB | | | SB | | |
| HCM Control Delay, s | | | | 0.7 | | | 32.1 | | | 23.6 | | |
| HCM LOS | 0.4 | | | 0.7 | | | JZ.1 | | | C C | | |
| TIONI LOG | fr at | | | 313 | | | U | | | U | | |
| Minor Lane/Major Musi | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WRD | SBLn1 | 2 | 18 | |
| Minor Lane/Major Mvr | III | | | EDI | EDIT | | VVDI | MOK | | _ | | |
| Capacity (veh/h) | | 197 | 1071 | • | - | 888 | - | - | 242 | | | |
| HCM Lane V/C Ratio | | 0.331 | 0.035 | | | 0.043 | | - | 0.201 | | | |
| HCM Control Delay (s | | 32.1 | 8.5 | | - | 9.2 | - | | 23.6 | | | |
| HCM Lane LOS | | D | A | - | | A | | | С | 1 | | |
| HCM 95th %tile Q(veh | 1) | 1.4 | 0.1 | - | - | 0.1 | - | - | 0.7 | | | |

| Intersection | | | | | | | | | | | | |
|--|--------|-----------|-------|--------|----------|-------|--------|-------|----------------|--------|-------|-------|
| Int Delay, s/veh | 7.2 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | † | 7 | 7 | † | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 19 | 339 | 20 | 48 | 571 | 16 | 32 | 1 | 51 | 40 | 2 | 46 |
| Future Vol, veh/h | 19 | 339 | 20 | 48 | 571 | 16 | 32 | 1 | 51 | 40 | 2 | 46 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | - | None | | | None | - | - 10 | None | | | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | - | - | - | - |
| Veh in Median Storage | e,# - | 0 | | - | 0 | - | - | 0 | | | 0 | - |
| Grade, % | - | 0 | - | | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 21 | 373 | 22 | 55 | 656 | 18 | 46 | 1 | 74 | 56 | 3 | 64 |
| | | | | | | | | | | | | |
| Major/Minor I | Major1 | | | Major2 | | 100 | Minor1 | 1 T | 1 | Minor2 | | |
| Conflicting Flow All | 674 | 0 | 0 | 395 | 0 | 0 | 1224 | 1199 | 373 | 1230 | 1203 | 656 |
| Stage 1 | | | - | | - | | 415 | 415 | 3 . | 766 | 766 | |
| Stage 2 | | | | | | | 809 | 784 | | 464 | 437 | |
| Critical Hdwy | 4.12 | - No. | 1 - | 4.12 | 1 10 0 | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | - | | | | | | 6.12 | 5.52 | | 6.12 | 5.52 | |
| Critical Hdwy Stg 2 | | | | | | 10- | 6.12 | 5.52 | | | 5.52 | - |
| Follow-up Hdwy | 2.218 | | | 2.218 | | - | 3.518 | | 3.318 | | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 917 | | | 1164 | - | 10. | 156 | 185 | 673 | 154 | 184 | 465 |
| Stage 1 | | | | | | - | 615 | 592 | | 395 | 412 | - |
| Stage 2 | | | | | - | - | 374 | 404 | | 578 | 579 | - |
| Platoon blocked, % | | | | | | - | | | | | | |
| Mov Cap-1 Maneuver | 917 | | | 1164 | - | 0 - | 126 | 172 | 673 | 129 | 171 | 465 |
| Mov Cap-2 Maneuver | - | | - | | | | 126 | 172 | - | 129 | 171 | - |
| Stage 1 | | | 1 1 | | - | | 601 | 578 | | 386 | 393 | . 90 |
| Stage 2 | - | - | - | | | | 305 | 385 | - | 501 | 566 | - |
| The state of the s | | | 1 | TW | | | | | | | 133 | |
| Approach | EB | | | WB | | | NB | 44 | | SB | 13 | |
| HCM Control Delay, s | 0.5 | | | 0.6 | The same | | 32.3 | | | 44 | | |
| HCM LOS | | | | | | | D | | | Е | | |
| | | | | 130 | | | TOP | | | | | |
| Minor Lane/Major Myn | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | | 1 |
| Capacity (veh/h) | | 250 | 917 | | 1 6 | 1164 | 1 | 7 102 | 209 | | | |
| HCM Lane V/C Ratio | | | 0.023 | | - | 0.047 | | | 0.585 | | | |
| Account to the second second second second | 1 | | 9 | | | 8.2 | - 10 | | 44 | | | |
| HCM Control Delay (s) | } | 32.3 | 9 | | | 0.2 | | | and the second | | | |
| HCM Control Delay (s) |) | 32.3 D | A | | | Α.2 | | | E | | | |

| Intersection | |
|--|------|
| Int Delay, s/veh 4.3 | |
| | SBR |
| Lane Configurations T T T T T T T | |
| | 25 |
| Future Vol, veh/h 34 660 14 50 434 41 23 2 37 13 1 | 25 |
| Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 | 0 |
| Sign Control Free Free Free Free Free Free Stop Stop Stop Stop Stop Stop Stop Stop | Stop |
| RT Channelized None No | lone |
| Storage Length 415 - 415 - 415 | - |
| Veh in Median Storage, # - 0 0 0 | |
| Grade, % - 0 0 0 | - |
| Peak Hour Factor 91 91 91 87 87 87 69 69 69 72 72 | 72 |
| Heavy Vehicles, % 2 2 2 2 2 2 2 2 2 2 2 | 2 |
| | 35 |
| | |
| Major/Minor Major1 Major2 Minor1 Minor2 | |
| | 499 |
| Stage 1 799 799 - 613 613 | |
| Stage 2 655 660 - 835 814 | |
| | 6.22 |
| Critical Hdwy Stg 1 6.12 5.52 - 6.12 5.52 | - |
| Critical Hdwy Stg 2 6.12 5.52 - 6.12 5.52 | |
| | .318 |
| | 572 |
| Stage 1 379 398 - 480 483 | - |
| Stage 2 455 460 - 362 391 | - |
| Platoon blocked, % | |
| Mov Cap-1 Maneuver 1023 867 93 116 425 86 122 5 | 572 |
| Mov Cap-2 Maneuver 93 116 - 86 122 | - |
| Stage 1 365 384 - 463 451 | - |
| Stage 2 398 430 - 303 377 | - |
| | |
| Approach EB WB NB SB | |
| HCM Control Delay, s 0.4 0.9 45 31 | |
| HCM LOS E D | |
| | |
| Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1 | |
| Capacity (veh/h) 176 1023 867 192 | |
| HCM Lane V/C Ratio 0.511 0.037 0.066 0.282 | |
| HCM Control Delay (s) 45 8.7 9.4 31 | 73.5 |
| HCM Lane LOS E A A D | |
| TIOW Lake LOS | |

| Intersection | | | | ý rosi | | | | | | | | |
|-------------------------|--------|----------|-------|--------|----------|-------|--------|-------|--------|--------|------|-------|
| Int Delay, s/veh | 7.3 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | † | 7 | 7 | † | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 19 | 342 | 20 | 48 | 578 | 16 | 32 | 1 | 51 | 40 | 2 | 46 |
| Future Vol, veh/h | 19 | 342 | 20 | 48 | 578 | 16 | 32 | 1 | 51 | 40 | 2 | 46 |
| Conflicting Peds, #/hr | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | | None | 10 . | 771 | None | | | None | | 113 | None |
| Storage Length | 415 | | 415 | 415 | | 415 | | | - | | | |
| Veh in Median Storage | | 0 | | Ville | 0 | | | 0 | - | | 0 | - |
| Grade, % | - | 0 | | | 0 | | - | 0 | | | 0 | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mymt Flow | 21 | 376 | 22 | 55 | 664 | 18 | 46 | 1 | 74 | 56 | 3 | 64 |
| | | | | | | | | | | | | |
| Major/Minor | Major1 | | | Major2 | | 100 | Minor1 | | -1 6 | Minor2 | ê în | |
| Conflicting Flow All | 682 | 0 | 0 | 398 | 0 | 0 | 1235 | 1210 | 376 | 1241 | 1214 | 664 |
| Stage 1 | - | | | | | 1 | 418 | 418 | - | 774 | 774 | |
| Stage 2 | | | | | | | 817 | 792 | | 467 | 440 | |
| Critical Hdwy | 4.12 | | | 4.12 | | 1 | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | | | | | | | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | | | | | | | 6.12 | 5.52 | 4 | 6.12 | 5.52 | 1 0- |
| Follow-up Hdwy | 2.218 | | | 2.218 | | | 3.518 | 4.018 | 3.318 | | | 3.318 |
| Pot Cap-1 Maneuver | 911 | | - | 1161 | - | | 153 | 183 | 670 | 152 | 182 | 461 |
| Stage 1 | - | | | - | | | 612 | 591 | - | 391 | 408 | - |
| Stage 2 | | | | 1 3. | - | | 370 | 401 | | 576 | 578 | - |
| Platoon blocked, % | | | - | | | - | | | | | | |
| Mov Cap-1 Maneuver | 911 | - | 177- | 1161 | | 1 | 123 | 170 | 670 | 127 | 169 | 461 |
| Mov Cap-2 Maneuver | | | | | - | - | 123 | 170 | - | 127 | 169 | - |
| Stage 1 | | - | | | | - | 598 | 577 | | 382 | 389 | 4 |
| Stage 2 | | | | | | | 301 | 382 | - | 499 | 565 | - |
| | 117 | 19 | | 1125 | 4 | | | | | | | 1 51 |
| Approach | EB | | | WB | | | NB | | | SB | L | |
| HCM Control Delay, s | 0.5 | | | 0.6 | | | 33.4 | | | 45.2 | | |
| HCM LOS | | | | | | | D | | | E | | |
| | 3.77 | | | | | | | 100 | | | | - 1 |
| Minor Lane/Major Mvi | mt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | 533 | | 10 |
| Capacity (veh/h) | | 245 | 911 | | | 1161 | | - | 206 | | | |
| HCM Lane V/C Ratio | | | 0.023 | | | 0.048 | | | 0.593 | | | |
| HCM Control Delay (s | () | 33.4 | 9 | - | | 8.3 | - | | 710000 | | | |
| HCM Lane LOS | , | D | Α | - | | Α | | | Е | | | |
| HCM 95th %tile Q(vel | h) | 2.5 | 0.1 | 75. | | | | | | | | |
| TOWN JOHN TOWNE CONTROL | 1) | 2.0 | 0,1 | 100 | 18.1 | 0.1 | | • | 0.0 | | | |

| Intersection | | | | | | | | | | | | |
|------------------------|--------|----------|-------|--------|----------|-------|--------|-------|-------|--------|-------|-------|
| Int Delay, s/veh | 4.4 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 1 | • | 7 | 7 | † | 7 | | स | | | 4 | |
| Traffic Vol, veh/h | 34 | 668 | 14 | 50 | 438 | 41 | 23 | 2 | 37 | 13 | 1 | 25 |
| Future Vol, veh/h | 34 | 668 | 14 | 50 | 438 | 41 | 23 | 2 | 37 | 13 | 1 | 25 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | - | None | | | None | | 13. | None | | | None |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | - | - | - | - |
| Veh in Median Storage | e,# - | 0 | | | 0 | | | 0 | - | | 0 | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 37 | 734 | 15 | 57 | 503 | 47 | 33 | 3 | 54 | 18 | 1 | 35 |
| | | | | | | | | | | | | |
| Major/Minor | Major1 | | - 10 | Major2 | 100 | - | Minor1 | 14.5 | - 0 | Minor2 | | |
| Conflicting Flow All | 550 | 0 | 0 | 749 | 0 | 0 | 1467 | 1472 | 734 | 1461 | 1440 | 503 |
| Stage 1 | | | | | | - | 808 | 808 | - 4 | 617 | 617 | |
| Stage 2 | | | | - | _ | - | 659 | 664 | | 844 | 823 | - |
| Critical Hdwy | 4.12 | | | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | | | | - | | | 6.12 | 5.52 | - | 6.12 | 5.52 | |
| Critical Hdwy Stg 2 | | | - | | | - | 6.12 | 5.52 | - | 6.12 | 5.52 | 100 |
| Follow-up Hdwy | 2.218 | | - | 2.218 | - | | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 1020 | - | | 860 | - | - | 106 | 127 | 420 | 107 | 133 | 569 |
| Stage 1 | - | - | | - | - | - | 375 | 394 | - | 477 | 481 | - |
| Stage 2 | | - | - | - | | - | 453 | 458 | - | 358 | 388 | - |
| Platoon blocked, % | | - | - | | - | - | | | | | | |
| Mov Cap-1 Maneuver | 1020 | | - | 860 | - | - | 91 | 114 | 420 | 85 | 120 | 569 |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 91 | 114 | - | 85 | 120 | - |
| Stage 1 | | | | | - | - 1 | 362 | 380 | | 460 | 449 | - |
| Stage 2 | - | - | | | - | - | 396 | 428 | - | 299 | 374 | - |
| | | | | | | | | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | a |
| HCM Control Delay, s | 0.4 | | | 0.9 | | | 46.3 | | | 31.3 | | |
| HCM LOS | | | | | | | Е | | | D | | |
| | | | | | | | | | | | | |
| Minor Lane/Major Mvr | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | _ 11 | |
| Capacity (veh/h) | | 173 | 1020 | | | 860 | -0 | | 190 | 1 | | |
| HCM Lane V/C Ratio | | | 0.037 | | | 0.067 | | | 0.285 | | | |
| HCM Control Delay (s |) | 46.3 | 8.7 | | | 9.5 | | | | 77 | | |
| HCM Lane LOS | 1 | Ε | A | | | A | | | D | | | |
| HCM 95th %tile Q(vet | 1) | 2.6 | 0.1 | E 27. | | 0.2 | | | 1.1 | | | |





Signal Warrant Sheets for Existing (2022) Conditions



WORST CASE DELAY FOR MINOR STREET APPROACH:

PEAK HOUR SIGNAL WARRANT ANALYSIS (Warrant #3, California MUTCD 2014 Edition, Rev 6)

INT #1

| PROJECT NAME: | Summer Brook | | | | | |
|----------------|--------------------------------|--------------|-----------------|---------------------|---|--|
| SCENARIO: | Existing Conditions PP | | | | | |
| COMMENTS: | | | | | | |
| MAJOR STREET: | Green Valley Road | ■ NB/SB | ☑ EB/WB | # OF APPROACH LANES | 1 | |
| MINOR STREET: | Deer Valley Road | ✓ NB/SB | ☐ EB/WB | # OF APPROACH LANES | 1 | |
| THE STUDY INTE | RSECTION HAS MORE THAN THREE | E APPROACH | HES (Y OR N): | | N | |
| ISOLATED COMM | JUNITY WITH POPULATION LESS TH | AN 10,000 (Y | OR N): | | N | |
| 85TH PERCENTIL | E SPEED GREATER THAN 40 MPH (| ON MAJOR S | TREET (Y OR N): | | Υ | |
| | | | АМ | PM | | |

| | | | MAJOR | STREET | | MINOR | STREET | | | |
|----------|----|----------|----------------|----------------|-------|----------------|----------------|-----------|-------|-----------------------|
| | | | EB Approach | WB Approach | Total | NB Approach | SB Approach | Heavy Leg | Total | Intersection Total |
| 06:00 AM | TO | 07:00 AM | | | 0 | | | 0 | 0 | 0 |
| 07:00 AM | TO | 08:00 AM | 340 | 589 | 929 | 36 | 66 | 66 | 102 | 1031 |
| 08:00 AM | то | 09:00 AM | | | 0 | | | 0 | 0 | 0 |
| 09:00 AM | TO | 10:00 AM | | | 0 | | | 0 | 0 | 0 |
| 10:00 AM | TO | 11:00 AM | | | 0 | | | 0 | 0 | 0 |
| 11:00 AM | TO | 12:00 PM | | | 0 | | | 0 | 0 | 0 |
| 12:00 PM | TO | 01:00 PM | | | 0 | | | 0 | 0 | 0 |
| 01:00 PM | то | 02:00 PM | | | 0 | | | 0 | 0 | 0 |
| 02:00 PM | ТО | 03:00 PM | | | 0 | | | 0 | 0 | 0 |
| 03:00 PM | TO | 04:00 PM | | | 0 | | | 0 | 0 | 0 |
| 04:00 PM | TO | 05:00 PM | | 1 | 0 | 5 | | 0 | 0 | 0 |
| 05:00 PM | TO | 06:00 PM | 593 | 494 | 1087 | 34 | 29 | 34 | 63 | 1150 |
| 06:00 PM | TO | 07:00 PM | | | 0 | | | 0 | 0 | 0 |
| 07:00 PM | то | 08:00 PM | | | 0 | | | 0 | 0 | 0 |
| 08:00 PM | TO | 09:00 PM | | | 0 | | | 0 | 0 | 0 |
| 09:00 PM | то | 10:00 PM | | | 0 | | | 0 | 0 | 0 |

22.6 sec/veh

28.5

| M. | AJOR STREET | | MINOR S | TREET | 11 | NTERSECTION |
|--------|-------------|--------|-----------|-------|--------|-------------|
| | Total | | Heavy Leg | Total | | Total |
| AM MAX | 929 | AM MAX | 66 | 102 | AM MAX | 1031 |
| PM MAX | 1087 | PM MAX | 34 | 63 | PM MAX | 1150 |

Kimley » Horn

Traffic Signal Warrants Worksheet

Warrant 3: Peak Hour

Source: California MUTCD 2014 Edition, Rev 6

Scenario: Existing Conditions PP AM

Intersection: Green Valley Road AND Deer Valley Road

Comments:

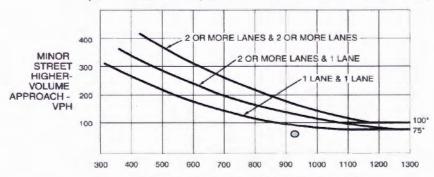
| | PART A or PART B | SATISFIED | NO | |
|---------------|--|-----------|-----|--|
| PART A | | | | |
| (All parts 1, | 2, and 3 below must be satisfied) | SATISFIED | NO | |
| 1. | The total delay experienced for traffic on one minor street approach controlled by a STOP sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle hours for a two-lane approach; AND | | No | |
| 2. | The volume on the same minor street approach equals or exceeds 10 vph for one moving lane of traffic or 150 vph for two moving lanes; <u>AND</u> | 0 | No | |
| 3. | The total entering volume serviced during the hour equals or exceeds 800 vph for intersection with four or more approaches or 650 vph for intersection with less than four approaches. | | Yes | |

PART B SATISFIED No

| | | 2 or |
|---------------------------------|-----|------|
| APPROACH LANES | One | More |
| Both Approaches - Major Street | 929 | |
| Highest Approach - Minor Street | 66 | |

The plotted points for vehicles per hour on major streets (both approaches) and the corresponding per hour higher volume minor street approach (one direction only) for one hour (any consecutive 15 minute period) fall above applicable curves in MUTCD Figure 4C-3.

Figure 4C-4. Warrant 3, Peak Hour (70% Factor)
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

Kimley » Horn

Traffic Signal Warrants Worksheet

Warrant 3: Peak Hour

Source: California MUTCD 2014 Edition, Rev 6

Scenario: Existing Conditions PP PM

Intersection: Green Valley Road AND Deer Valley Road

Comments:

| | | PART A or PART B | SATISFIED | NO | |
|----------|-------|--|-----------|-----|--|
| PART A | 4 | | | | |
| (All par | ts 1, | 2, and 3 below must be satisfied) | SATISFIED | NO | |
| | 1. | The total delay experienced for traffic on one minor street approach controlled by a STOP sign equals or exceeds four vehicle-hours for a one-lane approach and five vehicle hours for a two-lane approach; AND | | No | |
| | 2. | The volume on the same minor street approach equals or exceeds 100 vph for one moving lane of traffic or 150 vph for two moving lanes; AND | | No | |
| | 3. | The total entering volume serviced during the hour equals or exceeds 800 vph for intersection with four or more approaches or 650 vph for intersection with less than four approaches. | | Yes | |
| | | | | | |

PART B SATISFIED No

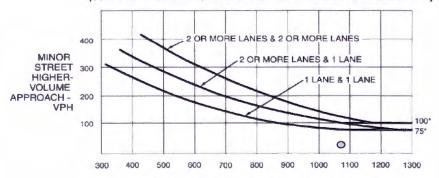
 APPROACH LANES
 One
 More

 Both Approaches - Major Street
 1087

 Highest Approach - Minor Street
 34

The plotted points for vehicles per hour on major streets (both approaches) and the corresponding per hour higher volume minor street approach (one direction only) for one hour (any consecutive 15 minute period) fall above applicable curves in MUTCD Figure 4C-3.

Figure 4C-4. Warrant 3, Peak Hour (70% Factor)
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.



Memorandum

To: Adam Bane, El Dorado County

From: Stephen Dillon, E.I.T.

Robert Paderna, P.E., RSP₁

Re: Summer Brook

Green Valley Road/Deer Valley Road Intersection Operations Analysis

Date: April 12, 2022

RECEIVED

MAR 2 0 2023

EL DORADO COUNTY
PLANNING AND BUILDING DEPARTMENT

The purpose of this memorandum is to document anticipated intersection operations at Green Valley Road and Deer Valley Road (the "study intersection") under both Near Term (2031) and Cumulative (2041) conditions, with and without the Summer Brook residential development project trips. Kimley-Horn previously conducted an analysis of this intersection as part of the Traffic Impact Analysis prepared for the Summer Brook development in February 2007. This supplemental analysis is intended to inform recent conversations between Blue Mountain Inc., (the "Client") and County regarding current operating conditions of the study intersection and the project's conditions of approval (COA).

Analysis Background

The project proposes to construct a total of 29 single-family (detached) homes. Access to the site will be provided via two full-access driveways along Green Valley Road, east of the study intersection. As part of the development review process, a traffic impact analysis (TIA) for the proposed project was completed by Kimley-Horn in February 2007. The 2007 TIA established 2025 as the Cumulative condition year for evaluation and concluded the study intersection satisfied California Manual on Uniform Traffic Control Devices (CA MUTCD) peak-hour signal warrants during both the AM and PM peak-hours under both no project and plus project conditions. Contributions to the project's fair share for signalizing the study intersection was established as a Condition of Approval (COA) by the County for the project.

As future year conditions established using El Dorado County's Travel Demand Model (TDM) have been updated since the 2007 TIA, the Client desired to reexamine the previously established signalization COA. As part of the COA, traffic volume-based warrants presented in the CA MUTCD were reviewed by Kimley-Horn utilizing traffic counts from January 2019 and May 2021 for the purpose of comparing results against the Cumulative 2025 peak-hour warrants produced for the 2007 TIA. The updated signal warrant evaluation using January 2019 and May 2021 data concluded that a traffic signal was not warranted for both no project and plus project scenarios at the study intersection under current traffic conditions.

In order to inform conversations with the County regarding consideration of traffic signalization of the study intersection in the future, the Client requested an updated traffic operations analysis be conducted under both no project and plus project conditions for Near Term (2031) and Cumulative (2041) scenarios.

Analysis Methodology

Level of Service Definitions

The level of service (LOS) of a facility is a qualitative measure used to describe operational conditions. LOS ranges from A, which represents minimal delay, to F, which represents heavy delay and a facility that is operating at or near its functional capacity. LOS for this study was determined using methods defined in the *Highway Capacity Manual (HCM)* 6th Edition.



Based on the evaluation of the abovementioned traffic signal warrants, it was determined that the warrant for a traffic signal at the Green Valley Road at Deer Valley Road intersection is not satisfied under Existing plus Project conditions. The results of the warrant evaluation is summarized in **Table 1** below. Summary sheets for the warrant evaluation are included as **Attachment B**.

Table 1: Traffic Signal Warrant Evaluation Summary

| | | Traffic Sig | nal Warrant | |
|---|----------------------------|----------------------------|------------------------|--------------------------------|
| Intersection | Warrant 1 8-hour volume | Warrant 2 4-hour volume | Warrant 3 Peak Hour | Warrant 4 Pedestrian Volume |
| Green Valley Road & Deer Valley Road | Not Satisfied | Not Satisfied | Not Satisfied | Not Satisfied |

Equitable Share Responsibility

Because it was determined that the traffic signal warrant is not satisfied, the County has requested that the project applicant determine the project's fair share percentage of the construction costs to install a traffic signal at the subject intersection. The project's fair share contribution was calculated based on the project's proportionate traffic contribution to the overall future traffic volumes at the subject intersection. Based on the Caltrans *Guide for the Preparation of Traffic Impact Studies* (2002), the fair share calculation for impacts at an intersection is calculated using the following equation:

$$P = T/(T_B - T_E)$$

Where:

P = The equitable share for the proposed project's traffic impact.

T = The vehicle trips generated by the project during the peak hour of subject intersection in vehicles per hour, vph.

 T_B = The forecasted traffic volume on impacted intersection at the time of general plan build-out, vph.

 T_E = The existing traffic volume on the impacted intersection plus other approved projects that will generate traffic that has yet to be constructed/opened, vph.

Based on proposed project trips and traffic volumes contained in the *Summer Brook Traffic Impact Analysis* (dated February 14, 2007), we calculated a <u>fair share of 3.9%</u> for the proposed project's traffic impact.

Summary and Recommendations

Based on the traffic signal warrant criteria contained in the *CMUTCD*, the Warrants 1 through 4 are not satisfied at the Green Valley Road at Deer Valley Road intersection under Existing plus Project conditions. Therefore, no traffic control or other intersection control modifications are required at this time. The proposed project's fair share contribution percentage of construction costs to install a traffic signal was calculated to be 3.9%, which is equivalent to approximately \$10,000 to \$12,000. Note that this represents an opinion of probable construction costs under current market conditions, and Kimley-Horn cannot guarantee accuracy of construction cost estimates in future years due to factors beyond our control.

Attachments:

Attachment A – Traffic Counts
Attachment B – Warrant Analysis Worksheets



Attachment A
Traffic Counts

National Data & Surveying Services Intersection Turning Movement Count

| City: F Control: 2 | lescue | | 155) 10 | | | | | Data - | Total | | | | Pr | Date: | 21-070070-0 5/26/2021 | 001 | |
|-----------------------|---------|--------------|-------------|---------|---------|-----------|---------|-----------|---------|----------|---------|---------|----------------|---------|--------------------------|-------|------------|
| NS/EW Streets: | - | Green Va | iley Rd | | | Green Val | | | | Deer Val | ley Rd | | Deer Valley Rd | | | | |
| NOON | | NORTHE | | | | SOUTHE | | | | EASTB | | | | WESTE | | | |
| NOON | 1 NL | 1 NT | NR | 0 NU | 1 SL | 1 ST | 1 SR | SU | 0 EL | ET ET | 0 ER | 0 EU | WL. | MT 1 | WR | WU | TOTA |
| 11:00 AM | 3 | 88 | 1 | 0 | 9 | 74 | 4 | 0 | 2 | 0 | 5 | 0 | 1 | 0 | 5 | 0 | 192 |
| 11:15 AM | 5 | 85 | 1 | 0 | 4 | 85 | 3 | 0 | 3 | 0 | 4 | 0 | 2 | 1 | 6 | 0 | 199 |
| 11:30 AM | 2 | 83 | 3 | 0 | 6 | 91 | 5 | 0 | 5 | 0 | 7 | 0 | 1 | 0 | 5 | 0 | 208 |
| 11:45 AM | 3 | 108 | 2 | 0 | 2 | 79 | 4 | 0 | 3 | 1 | 5 | 0 | 4 | 1 | 8 | 0 | 220 |
| 12:00 PM | 4 | 81 | 4 | 0 " | 4 | 86 | 6 | 0 | 1 | 0 | 2 | 0 | 2 | 0 | 13 | 0 | 203 |
| 12:15 PM | 7 | 96 | 0 | 2 | 6 | 93 | 3 | 0 | 7 | 0 | 5 | 0 | 1 | 0 | 6 | 0 | 226 |
| 12:30 PM | 2 | 98 | 0 | 0 | 5 | 89 | 5 | 0 | 4 | 0 | 2 | 0 | 2 | 0 | 10 | 0 | 217 |
| 12:45 PM | 4 | 108 | 4 | 0 | 5 | 102 | 4 | 0 | 6 | 0 | 3 | 0 | 0 | 0 | 9 | 0 | 245 |
| 1:00 PM | 4 | 97 | 8 | 0 | 10 | 105 | 6 | 0 | 1 | 1 | 3 | 0 | 4 | 1 | 4 | 0 | 244 |
| 1:15 PM | 11 | 93 | 6 | 0 | 5 | 113 | 2 | 0 | 3 | 0 | 3 | 0 | 2 | 0 | 1 | 0 | 239 |
| 1:30 PM | 2 | 94 | 3 | 1 | 9 | 99 | 7 | o | 5 | 0 | 3 | 0 | 4 | 0 | 5 | 0 | 237 |
| 1:45 PM | 8 | 96 | 1 | ō | 11 | 88 | 5 | 0 | 4 | 1 | 6 | 0 | 1 | 2 | 1 | 0 | 224 |
| | NL | NT | NR | NU | SL | ST | SR | SU | EL | ET | ER | EU | WL. | WT | WR | WU | TOT |
| TOTAL VOLUMES: | 55 | 1127 | 33 | 3 | 76 | 1104 | 54 | 0 | 44 | 3 | 48 | 0 | 24 | 5 | 73 | 0 | 264 |
| APPROACH %'s: | 4.52% | 92.53% | 2.71% | 0.25% | 6.16% | 89.47% | 4.38% | 0.00% | 46.32% | 3.16% | 50.53% | 0.00% | 23.53% | 4.90% | 71.57% | 0.00% | |
| PEAK HR: | | 12:45 PM - | | | | | | | | | | | | | | | TOT |
| PEAK HR VOL : | 21 | 392 | 21 | 1 | 29 | 419 | 19 | 0 | 15 | 1 | 12 | 0 | 10 | 1 | 19 | 0 | 960 |
| PEAK HR FACTOR : | 0.477 | 0.907 | 0.656 | 0.250 | 0.725 | 0.927 | 0.679 | 0.000 | 0.625 | 0.250 | 1.000 | 0.000 | 0.625 | 0.250 | 0.528 | 0.000 | 0.98 |
| | | 0.93 | 38 | | | 0.96 | i5 | - deleted | | 0.77 | 78 | | | 0.8 | 33 | | 0.50 |
| | | NORTHI | BOUND | | | SOUTHE | BOUND | | | EASTB | OUND | | | WESTE | BOUND | | |
| PM | 1 NL | 1 NT | 1 NR | 0 NU | 1 SL | 1 ST | 1 SR | O SU | 0 EL | 1 ET | 0 ER | 0 EU | 0 WL | 1 WT | 0 WR | WU | TOTA |
| 2:00 PM | 4 | 93 | 1 | 0 | 7 | 126 | 0 | 0 | 6 | 1 | 8 | 0 | 2 | 0 | 5 | 0 | 253 |
| 2:15 PM | 9 | 86 | 7 | 1 | 7 | 124 | 9 | o | 3 | o | 3 | 0 | 1 | 0 | 4 | 0 | 254 |
| | | | 7 | | , | | - | 0 | 3 | 0 | 3 | 0 | 5 | 0 | 7 | o | 212 |
| 2:30 PM | 3 | 77 | | 0 | 2 | 105 | 2 | • | | - | 1 | - | | 0 | | 0 | 247 |
| 2:45 PM | 4 | 90 | 2 | . 0 | 5 | 127 | 2 | . 0. | 6 | 0 | 0 | 0 | 2 . | | 4 | | 23 |
| 3:00 PM | 2 | 79 | 6 | 0 | 8 | 129 | 1 | 0 | 3 | 0 | 2 | 0 | 1 | 0 | 6 | 0 | |
| 3:15 PM | 5 | 99 | 2 | 0 | 8 | 127 | 3 | 0 | 6 | 0 | 8 | 0 | 0 | 0 | 4 | 0 | 26 |
| 3:30 PM | 3 | 82 | 1 | 0 | 9 | 116 | 5 | 1 | 8 | 1 | 2 | 0 | 4 | 0 | 11 | 0 | 243 |
| 3:45 PM | 7 | 99 | . 1 | . 0 | 10 | 128 | 3 | . 0 | 4 | . 0 | 6 | 0 | 7 | 0 | | 0 | 27 |
| 4:00 PM | 5 | 129 | 2 | 0 | 8 | 145 | 2 | 0 | 4 | 0 | 3 | 0 | 3 | 0 | 3 | 0 | 304 |
| 4:15 PM | 5 | 105 | 3 | 0 | 7 | 127 | 4 | 0 | 5 | 1 | 6 | 0 | 3 | 0 | 6 | 0 | 272 |
| 4:30 PM | 4 | 103 | 5 | 0 | 7 | 103 | 5 | 0 | 4 | 0 | 3 | 0 | 3 | 1 | 5 | 0 | 243 |
| 4:45 PM | 5 | 86 | 4 | 0 | . 5 | 134 | 9 | 0 | .3 | 0 | 6 | .1. | 4 | 0 | 6 | 0 | 263 |
| 5:00 PM | 10 | 94 | 7 | 0 | 2 | 111 | 6 | 0 | 5 | 0 | 5 | 0 | 0 | 0 | 7 | 0 | 247 |
| 5:15 PM | 1 | 85 | 5 | 0 | 7 | 170 | 5 | 0 | 2 | 0 | 1 | 0 | 4 | 0 | 6 | 0 | 280 |
| 5:30 PM | 4 | 104 | 2 | 0 | 7 | 135 | 5 | 0 | 5 | 0 | 4 | 0 | 4 | 0 | 3 | 0 | 27. |
| 5:45 PM | 3 | 99 | 3 | 0 | 8 | 122 | 3 | 0 | 1 | 0 | 6 | 0 | 2 | 0 | . 7 | . 0 | 25 |
| 6:00 PM | 6 | 85 | 1 | 1 | 6 | 105 | | 0 | 2 | 0 | 5 | 0 | 2 | 0 | 0 | 0 | 218 |
| 6:15 PM | 5 | 73 | 4 | 0 | 6 | 77 | 3 | 0 | 3 | 0 | 5 | 0 | 1 | 0 | 5 | 0 | 183 |
| 6:30 PM | 0 | 65 | 3 | 0 | 5 | 97 | 2 | 0 | 3 | 0 | 2 | 0 | 0 | 0 | 5 | 0 | 182 |
| 6:45 PM | 2 | 38 | 3 | 0 | 4 | 93 | 3 | 0 | 2 | 0 | 5 | 0 | 0 | 1 | 2 | 0 | 153 |
| | NL | NT | NR | NU | SL | ST | SR | SU | ÉL | ET | ER | EU | WL | WT | WR | WU | TOT |
| TOTAL VOLUMES : | 87 | 1771 | 69 | 2 | 128 | 2401 | 77 | 1 | 78 | 3 | 81 | 1 | 48 | 2 | 103 | 0 | 485 |
| APPROACH %'s : | 4.51% | 91.81% | 3.58% | 0.10% | 4.91% | 92.10% | 2.95% | 0.04% | 47.85% | 1.84% | 49.69% | 0.61% | 31.37% | 1.31% | 67.32% | 0.00% | |
| PEAK HR: | | 03:45 PM - | | 0 | 22 | 503 | | 0 | 17 | | 18 | 0 | 16 | 1 | 21 | 0 | TOT 109 |
| PEAK HR VOL: | 21 | 436 0.845 | 11 0.550 | 0 | 32 | | 14 | | | 1 | | | | | | | 109 |
| PEAK HR FACTOR : | 0.750 | | | 0.000 | 0.800 | 0.867 | 0.700 | 0.000 | 0.850 | 0.250 | 0.750 | 0.000 | 0.571 | 0.250 | 0.750 | 0.000 | 0.89 |

Tuesday 5/11/2021

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| | 113 | 458 | 64 | 260 | 3 | 29 | 5 | 38 | 185 | 785 | | 25 | 128 | 46 | 232 | 5 | 17 | 4 | 12 | 80 | 389 |
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| Р | | 54.3% | | 36.9% | | 2.9% | | 5.9% | | | Р | | 38.1% | | 54.2% | | 3.8% | | 3.9% | | |
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| Р | | 0.869 875 | | 0.928 507 | | 0.818 | | 0.620 96 | | | Р | | 0.894 685 | | 0.934 996 | | 0.732 68 | | 0.783 | | |
| P | | 07:45 | | 07:45 | | 07:45 | | 07:15 | | | Р | | 16:00 | | 16:45 | | 16:15 | | 16:00 | | |
| P | | 466 0.869 | | 286 0.794 | | 36 0.818 | | 62 0.620 | | | P | | 378 0.955 | | 509 0.957 | | 41 0.732 | | 34 0.850 | | |



Attachment B Warrant Analysis Worksheet

TRAFFIC SIGNAL VOLUME WARRANT ANALYSIS (2012 MUTCD)

| MAJOR STREET: | Green Valley Road | NB | SB | # OF APPROACH LANES: | 1 |
|---------------|---|-------------|----|----------------------|---|
| MINOR STREET: | Deer Valley Road | EB | WB | # OF APPROACH LANES: | 1 |
| CITY, STATE: | El Dorado County, CA | | | | |
| COMMENTS: | Existing Conditions | | | | |
| | WITH POPULATION LESS THAN 10 SPEED GREATER THAN 40 MPH ON MA | , , , , , , | | N Y | |

| | | | MAJOR ST | MINOR ST | Ped Count | WARRAN | 1 - Conditi | on A, Part 1 | WARRANT | 1 - Condition | on B, Part 1 | WARRANT | 1 - Conditi | on A, Part 2 | WARRANT | 1 - Conditio | on B, Part 2 | WARRANT 2 | WARRANT 3 | WARR | ANT 4 |
|----------|--------|----------|--------------------|----------------------|----------------------|-----------|-------------|--------------|-----------|---------------|--------------|-----------|-------------|--------------|----------------|----------------|--------------|---------------|-------------|---------------------------|------------------------|
| | | | TWO-WAY TRAFFIC | TRAFFIC HEAVY LEG | CROSSING MAJOR ST | MAIN LINE | SIDE | вотн мет | MAIN LINE | SIDE | BOTH MET | MAIN LINE | SIDE | BOTH MET | MAIN LINE | SIDE STREET | BOTH MET | Four-Hour | Peak Hour | Ped Volume (Four-hour) | Ped Volum (Peak-hou |
| HRESHOLD | VALUES | 5 | | | | 350 | 105 | | 525 | 53 | | 280 | 84 | | 420 | 42 | | 60 | 75 | 100 | 190 |
| 02:00 AM | TO | 03:00 AM | | | | | | | | | | | | | | | | | | | |
| 03:00 AM | TO | 04:00 AM | | | | | | | | | | | | | | | | | | | |
| 04:00 AM | TO | 05:00 AM | | | | | | | | | | | | | | | | | | | |
| 05:00 AM | TO | 06:00 AM | | | | | | | | | | | | | | | | | | | |
| 06:00 AM | TO | 07:00 AM | | | | | | | | | | | | | | | | | | | |
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| 09:00 AM | TO | 10:00 AM | | | | | | | | | | | | | | | | | | | |
| 10:00 AM | TO | 11:00 AM | | | | | | | | | | | | | | | | | | | |
| 11:00 AM | TO | 12:00 PM | 750 | 35 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 12:00 PM | TO | 01:00 PM | 818 | 43 | 0 | Y | | | Y | | | Y | | | Y | Y | Y | | | | |
| 01:00 PM | TO | 02:00 PM | 884 | 30 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 02:00 PM | TO | 03:00 PM | 900 | 31 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 03:00 PM | TO | 04:00 PM | 934 | 40 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 04:00 PM | TO | 05:00 PM | 1,012 | 36 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 05:00 PM | TO | 06:00 PM | 998 | 33 | 0 | Y | | | Y | | | Y | | | Y | | | | | | |
| 06:00 PM | TO | 07:00 PM | 692 | 27 | 0 | Y | | | Y | | | Y | | | Y | | | | | | - |
| 07:00 PM | TO | 08:00 PM | | | | | | | | | | | | | | | | | | | |
| 08:00 PM | TO | 09:00 PM | | | | | | | | | | | | | | | | | | | |
| 09:00 PM | TO | 10:00 PM | | | | | | | | | | | | | | | | | | | |
| | | | 6,988 | 275 | | 8 | 0 | 0 | 8 | 0 | . 0 | 8 | 0 | 0 | 8 | 1 | 1 | 0 | 0 | 0 | 0 |
| | | | | | | | IOURS NEE | | | OURS NEE | | | 8 HOURS | | or both Condit | ion A & B | | 4 HRS NEEDED | 1 HR NEEDED | 4 HRS NEEDED | 1 HR NEED |
| | | | | | | N | OT SATISF | IED | N | OT SATISFI | ED | | | NOT SA | TISFIED | | | NOT SATISFIED | SATISFIED | SATISFIED | SATISFI |



Memorandum

To: Adam Bane, El Dorado County

From: Stephen Dillon, E.I.T.

Robert Paderna, P.E., RSP₁

Re: Summer Brook

Green Valley Road/Deer Valley Road Intersection Operations Analysis

Date: April 12, 2022

The purpose of this memorandum is to document anticipated intersection operations at Green Valley Road and Deer Valley Road (the "study intersection") under both Near Term (2031) and Cumulative (2041) conditions, with and without the Summer Brook residential development project trips. Kimley-Horn previously conducted an analysis of this intersection as part of the Traffic Impact Analysis prepared for the Summer Brook development in February 2007. This supplemental analysis is intended to inform recent conversations between Blue Mountain Inc., (the "Client") and County regarding current operating conditions of the study intersection and the project's conditions of approval (COA).

Analysis Background

The project proposes to construct a total of 29 single-family (detached) homes. Access to the site will be provided via two full-access driveways along Green Valley Road, east of the study intersection. As part of the development review process, a traffic impact analysis (TIA) for the proposed project was completed by Kimley-Horn in February 2007. The 2007 TIA established 2025 as the Cumulative condition year for evaluation and concluded the study intersection satisfied California Manual on Uniform Traffic Control Devices (CA MUTCD) peak-hour signal warrants during both the AM and PM peak-hours under both no project and plus project conditions. Contributions to the project's fair share for signalizing the study intersection was established as a Condition of Approval (COA) by the County for the project.

As future year conditions established using El Dorado County's Travel Demand Model (TDM) have been updated since the 2007 TIA, the Client desired to reexamine the previously established signalization COA. As part of the COA, traffic volume-based warrants presented in the CA MUTCD were reviewed by Kimley-Horn utilizing traffic counts from January 2019 and May 2021 for the purpose of comparing results against the Cumulative 2025 peak-hour warrants produced for the 2007 TIA. The updated signal warrant evaluation using January 2019 and May 2021 data concluded that a traffic signal was not warranted for both no project and plus project scenarios at the study intersection under current traffic conditions.

In order to inform conversations with the County regarding consideration of traffic signalization of the study intersection in the future, the Client requested an updated traffic operations analysis be conducted under both no project and plus project conditions for Near Term (2031) and Cumulative (2041) scenarios.

Analysis Methodology

Level of Service Definitions

The level of service (LOS) of a facility is a qualitative measure used to describe operational conditions. LOS ranges from A, which represents minimal delay, to F, which represents heavy delay and a facility that is operating at or near its functional capacity. LOS for this study was determined using methods defined in the *Highway Capacity Manual (HCM)* 6th Edition.



Intersection Analysis

The HCM includes procedures for analyzing side-street stop controlled (SSSC) intersections. The SSSC procedure defines LOS as a function of average control delay for each minor street approach movement. Table 1 presents intersection LOS definitions as defined in the HCM.

Table 1 - Intersection Level of Service Criteria

| Level of | Un-Signalized |
|------------------|----------------------------------|
| Service (LOS) | Average Control Delay* (sec/veh) |
| Α | ≤ 10 |
| В | > 10 - 15 |
| С | > 15 – 25 |
| D | > 25 – 35 |
| Е | > 35 – 50 |
| F | > 50 |

Source: Highway Capacity Manual, 6th Edition

LOS for the study intersections was determined using the Synchro® traffic analysis software. Synchro is an interactive computer program that enables planners and engineers to: forecast the traffic impacts of new developments; conduct area-wide traffic forecasting studies; test different mitigation measures and compare different traffic scenarios. Synchro 11 utilizes HCM 6 methodology to analyze intersection delay and LOS. Level of service for the Intersection is evaluated against El Dorado County thresholds of LOS D for Rural Regions¹.

Analysis Results

Synchro 11 analysis was conducted for the Intersection under Near Term (2031) and Cumulative (2041) no project and plus project conditions using present day intersection geometry. The results of the analysis are reported in Table 2.

The Intersection operates a satisfactory level for the El Dorado County Rural Region under all no project and plus project Near Term scenarios. While the Intersection operates at a deficient level for side street stop control under plus project Cumulative conditions, the Intersection is shown to be deficient under no project conditions as well. The project is shown to add a nominal level to delay to the intersection.

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^{*} Applied to the worst lane/lane group(s) for SSSC

¹ Transportation Impact Study Guidelines, El Dorado County Community Development Agency, November 2014.

Kimley » Horn

Table 2 - Intersection Levels of Service (Green Valley Rd/Deer Valley Rd)

| Scen | ario | LOS Threshold | Peak Hour | Delay (s) | LOS |
|------------|---------------|------------------|--------------|-----------|------|
| | No Project | | AM | 4.7(29.3) | A(D) |
| Near Term | No Project | | PM | 2.8(31.3) | A(D) |
| (2031) | Plus Project | | AM | 4.7(30.1) | A(D) |
| | Plus Project | | PM | 2.8(32.1) | A(D) |
| | No Project | D | AM | 7.2(44.0) | A(E) |
| Cumulative | No Project | | PM | 4.3(45.0) | A(E) |
| (2041) | Diva Designat | | AM | 7.3(45.2) | A(E) |
| | Plus Project | | PM | 4.4(46.3) | A(E) |

Note: Bold represents deficient operations.

Side Street Stop Control (SSSC) reported as intersection delay followed by worst approach's delay

Attachments:

Exhibit 1 - Project Vicinity Diagram

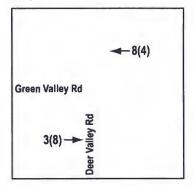
Attachment 1 - Analysis Worksheets for Near Term Conditions

Attachment 2 – Analysis Worksheets for Near Term plus Project Conditions

Attachment 3 - Analysis Worksheets for Cumulative Conditions

Attachment 4 - Analysis Worksheets for Cumulative plus Project Conditions

Project Trip Assignment



Near Term (2031) Level of Service

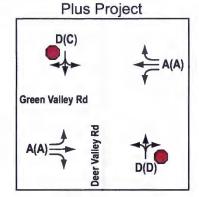
Organization (CD)

A(A)

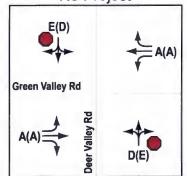
By A(A)

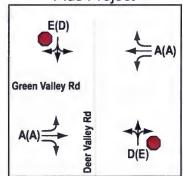
Green Valley Rd

C(D)

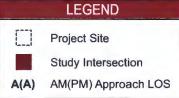


Cumulative (2041) Level of Service No Project Plus Project









Intersection LOS threshold for Rural Regions is LOS D per El Dorado County Transportation Impact Study Guidelines





Attachment 1

Analysis Worksheets for Near Term Conditions

| Intersection | | | | | | | | | | | | | |
|------------------------|--------|----------|------|--------|----------|-------|--------|-------|-------|--------|--------|-------|----------------|
| Int Delay, s/veh | 4.7 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | 1 | * | 7 | 7 | * | 7 | | 4 | | | 4 | | |
| Traffic Vol, veh/h | 17 | 320 | 15 | 32 | 545 | 14 | 27 | 1 | 38 | 34 | 1 | 42 | |
| Future Vol, veh/h | 17 | 320 | 15 | 32 | 545 | 14 | 27 | 1 | 38 | 34 | 1 | 42 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop | |
| RT Channelized | | - | None | | + | None | | | None | - | - | None | |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | - | - | - | - | |
| Veh in Median Storage | ,# - | 0 | - | | 0 | - | | 0 | - | | 0 | ٠ | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 19 | 352 | 16 | 37 | 626 | 16 | 39 | 1 | 55 | 47 | 1 | 58 | |
| | | | | | | | | | | | | | |
| Major/Minor I | Major1 | | 1 | Major2 | - | | Minor1 | | | Minor2 | | | |
| Conflicting Flow All | 642 | 0 | 0 | 368 | 0 | 0 | 1128 | 1106 | 352 | 1126 | 1106 | 626 | |
| Stage 1 | | | - | | | - | 390 | 390 | | 700 | 700 | - | |
| Stage 2 | | - | | - | - | - | 738 | 716 | - | 426 | 406 | | |
| Critical Hdwy | 4.12 | - 1- | - | 4.12 | - | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | |
| Critical Hdwy Stg 1 | - | - | | - | | | 6.12 | 5.52 | | 6.12 | 5.52 | - | |
| Critical Hdwy Stg 2 | | - | | | 1100 | 1 | 6.12 | 5.52 | | 6.12 | 5.52 | - | |
| Follow-up Hdwy | 2.218 | - | - | 2.218 | | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | |
| Pot Cap-1 Maneuver | 943 | | | 1191 | | | 181 | 210 | 692 | 182 | 210 | 484 | |
| Stage 1 | | - | | _ | | | 634 | 608 | - | 430 | 441 | - | |
| Stage 2 | | - | | | - | | 410 | 434 | | 606 | 598 | - | |
| Platoon blocked, % | | - | | | - | - | | | | | | | |
| Mov Cap-1 Maneuver | 943 | | - | 1191 | | | 152 | 200 | 692 | 160 | 200 | 484 | |
| Mov Cap-2 Maneuver | - | | - | - | | - | 152 | 200 | | 160 | 200 | | |
| Stage 1 | | 50. | | - | | | 621 | 596 | 100 | 421 | 427 | | |
| Stage 2 | - | - | | | | - | 348 | 421 | - | 545 | 586 | | |
| Lyr (In In Inc.) | | | | | | | | | | | -1111- | | |
| Approach | EB | | | WB | 511 | | NB | | | SB | AT BE | | |
| HCM Control Delay, s | 0.4 | | | 0.4 | | | 24.6 | | | 29.3 | | | Bridge Control |
| HCM LOS | | | | | | | C | | | D | | | |
| | | | | | | | | | | | | | |
| Minor Lane/Major Mvn | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | | * | |
| Capacity (veh/h) | | 278 | 943 | | - | 1191 | 1 . | | 253 | | | | |
| HCM Lane V/C Ratio | | 0.344 | 0.02 | | | 0.031 | | | 0.423 | | | | |
| HCM Control Delay (s |) | 24.6 | 8.9 | | | 8.1 | | 100. | | | | | |
| HCM Lane LOS | | C | A | - | | A | | | D | | | | |
| | 1) | 1.5 | 0.1 | | | 0.1 | | | 2 | | | | |

| Intersection | | - 1 | E L | | | | | | | | | | |
|--|---------|-----------|----------|--------|---------|----------|--------|-------|---------|--------|-------|---------|--|
| Int Delay, s/veh | 2.8 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | ħ | ^ | 7 | 7 | | 7 | | 4 | | | 4 | | |
| Traffic Vol, veh/h | 34 | 626 | 14 | 33 | 397 | 28 | 18 | 2 | 25 | 11 | 1 | 23 | |
| Future Vol, veh/h | 34 | 626 | 14 | 33 | 397 | 28 | 18 | 2 | 25 | 11 | 1 | 23 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop | |
| RT Channelized | | | None | | | None | | | None | | | None | |
| Storage Length | 415 | | 415 | 415 | | 415 | | | - | | - | - | |
| Veh in Median Storage | ,# - | 0 | | | 0 | | | 0 | | | 0 | - | |
| Grade, % | - | 0 | - | | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 37 | 688 | 15 | 38 | 456 | 32 | 26 | 3 | 36 | 15 | 1 | 32 | |
| | | | | | | | | | | | | | |
| Major/Minor N | //ajor1 | | 1 | Major2 | | 1 | Minor1 | T V | 1 | Minor2 | 1500 | WEST TO | |
| Conflicting Flow All | 488 | 0 | 0 | 703 | 0 | 0 | 1327 | 1326 | 688 | 1321 | 1309 | 456 | |
| Stage 1 | - | - | | | | - | 762 | 762 | - | 532 | 532 | - | |
| Stage 2 | | - | | | | | 565 | 564 | | 789 | 777 | | |
| Critical Hdwy | 4.12 | | -04 | 4.12 | | 10- | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | |
| Critical Howy Stg 1 | - | | - | | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - | |
| Critical Hdwy Stg 2 | | | - 6 | | | | 6.12 | 5.52 | | 6.12 | 5.52 | | |
| | 2.218 | | | 2.218 | - | | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | |
| Pot Cap-1 Maneuver | 1075 | - | | 895 | 110 | 4 | 132 | 156 | 446 | 134 | 159 | 604 | |
| Stage 1 | - | - | | - | - | - | 397 | 414 | - | 531 | 526 | | |
| Stage 2 | | | | | 1. | - | 510 | 508 | | 384 | 407 | | |
| Platoon blocked, % | | | - | | - | - | | | | | | | |
| Mov Cap-1 Maneuver | 1075 | - | | 895 | - 1 | - | 117 | 144 | 446 | 114 | 147 | 604 | |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 117 | 144 | - | 114 | 147 | | |
| Stage 1 | - | | - | | | | 384 | 400 | | 513 | 504 | - | |
| Stage 2 | - | - | - | - | - | - | 461 | 487 | - | 338 | 393 | | |
| | | | | | | | | | | | | | |
| Approach | EB | | | WB | | ERE | NB | E E | | SB | LLD. | | |
| HCM Control Delay, s | 0.4 | | | 0.7 | | | 31.3 | | | 23 | 1715 | 118 | |
| HCM LOS | | | | | | | D | | | С | | | |
| The state of the s | | | | | | | | | | 81 | | | |
| Minor Lane/Major Mym | it | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | | For | |
| Capacity (veh/h) | | 201 | 1075 | 11. | | 895 | | | 248 | | | T E TH | |
| HCM Lane V/C Ratio | | | 0.035 | - | - | 0.042 | - | - | 0.196 | | | | |
| | | | | | | | | | | | | | |
| | | 31.3 | 8.5 | - | - | 9.2 | - | - | 23 | | | | |
| HCM Control Delay (s) HCM Lane LOS | | 31.3 D | 8.5 A | | | 9.2 A | | | 23 C | | | 2000 | |





Analysis Worksheets for Near Term plus Project Conditions

| Intersection | | | | | | | | | | | | | |
|------------------------|--------|-------|------|--------|----------|------|--------|-------|----------------|--------|-------|-------|---------|
| Int Delay, s/veh | 4.7 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | 7 | 1 | 7 | 7 | † | 7 | | 4 | | | 4 | | |
| Traffic Vol, veh/h | 17 | 323 | 15 | 32 | 553 | 14 | 27 | 1 | 38 | 34 | 1 | 42 | |
| Future Vol, veh/h | 17 | 323 | 15 | 32 | 553 | 14 | 27 | 1 | 38 | 34 | 1 | 42 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop | |
| RT Channelized | | | None | - | | None | | | None | - | | None | |
| Storage Length | 415 | - | 415 | 415 | | 415 | - | - | - | - | - | - | |
| Veh in Median Storage | ,# - | 0 | 1 - | | 0 | | | 0 | | | 0 | | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 19 | 355 | 16 | 37 | 636 | 16 | 39 | 1 | 55 | 47 | 1 | 58 | |
| | | | | | | | | | | | | | |
| Major/Minor 1 | Major1 | | 1 | Major2 | | 1 | Minor1 | 444 | | Minor2 | | | , |
| Conflicting Flow All | 652 | 0 | 0 | 371 | 0 | 0 | 1141 | 1119 | 355 | 1139 | 1119 | 636 | |
| Stage 1 | - | - | | | | | 393 | 393 | - | 710 | 710 | - | |
| Stage 2 | - | - | | - | | | 748 | 726 | | 429 | 409 | - | |
| Critical Hdwy | 4.12 | - | | 4.12 | - 0. | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - | |
| Critical Hdwy Stg 2 | - | | | | - | - | 6.12 | 5.52 | | 6.12 | 5.52 | | |
| Follow-up Hdwy | 2.218 | - | - | 2.218 | - | - | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | |
| Pot Cap-1 Maneuver | 935 | - | | 1188 | | | 178 | 207 | 689 | 178 | 207 | 478 | |
| Stage 1 | - | - | - | - | - | - | 632 | 606 | - | 424 | 437 | - | |
| Stage 2 | | | | - | | | 404 | 430 | - 1- | 604 | 596 | | |
| Platoon blocked, % | | - | - | | - | - | | | | | | | |
| Mov Cap-1 Maneuver | 935 | | | 1188 | | | 149 | 197 | 689 | 156 | 197 | 478 | |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 149 | 197 | - | 156 | 197 | - | |
| Stage 1 | - | | | | | | 619 | 594 | | 416 | 423 | 1 | |
| Stage 2 | | - | - | - | - | - | 343 | 417 | | 543 | 584 | - | |
| | | | | | | | | | | | | | |
| Approach | EB | | W. | WB | | | NB | din's | | SB | | | Laure I |
| HCM Control Delay, s | 0.4 | | 1000 | 0.4 | | | 25.1 | | | 30.1 | 7. 1 | | |
| HCM LOS | | | | | | | D | | | D | | | |
| | | | | | | | | | | | | | |
| Minor Lane/Major Myn | nt i | VBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | | | 1 | |
| Capacity (veh/h) | | 273 | 935 | - | | 1188 | | | 248 | | | | |
| HCM Lane V/C Ratio | | 0.35 | 0.02 | - | | | | | 0.431 | | | | |
| HCM Control Delay (s) | | 25.1 | 8.9 | | | 8.1 | 14 | 1 | and the street | | | | |
| HCM Lane LOS | | D | Α | - | - | Α | - | - | D | | | | |
| HCM 95th %tile Q(veh | 1 | 1.5 | 0.1 | 0.5 | - | 0.1 | 1 1 | | 2 | | | | |

| ntersection | _ | | | | | | | | | | | | |
|---------------------------|--------|----------|-------|--------|----------|--------|--------------|--------------|---------|--------|-------------------|--|--|
| Int Delay, s/veh | 2.8 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| ane Configurations | M | * | 7 | 1 | ^ | 7 | | 4 | | | 4 | | |
| Traffic Vol, veh/h | 34 | 634 | 14 | 33 | 401 | 28 | 18 | 2 | 25 | 11 | 1 | 23 | |
| Future Vol, veh/h | 34 | 634 | 14 | 33 | 401 | 28 | 18 | 2 | 25 | 11 | 1 | 23 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | . 0 | |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop | |
| RT Channelized | - | 1 | None | | - | None | | | None | 177 | | None | |
| Storage Length | 415 | - | 415 | 415 | | 415 | | - | | | - | - | |
| Veh in Median Storage | | 0 | - | | 0 | | | 0 | | | 0 | 1 | |
| Grade, % | - | 0 | - | - | 0 | | | 0 | | | 0 | | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mymt Flow | 37 | 697 | 15 | 38 | 461 | 32 | 26 | 3 | 36 | 15 | 1 | 32 | |
| William Ion | 01 | 001 | 10 | 00 | 101 | 02 | 20 | | | 10 | | OZ. | |
| Major/Minor I | Major1 | | | Major2 | SELVE | | Minor1 | - 3 | 10-51 | Minor2 | - 5 | | |
| Conflicting Flow All | 493 | 0 | 0 | 712 | 0 | 0 | 1341 | 1340 | 697 | 1335 | 1323 | 461 | And the state of t |
| Stage 1 | - | - | U | 112 | - | - | 771 | 771 | 031 | 537 | 537 | 401 | |
| Stage 2 | | - | _ | _ | | | 570 | 569 | | 798 | 786 | | |
| Critical Hdwy | 4.12 | | | 4.12 | | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | |
| Critical Hdwy Stg 1 | | | | 4.12 | | | 6.12 | 5.52 | 0.22 | 6.12 | 5.52 | 0.22 | |
| , , | - | | | | | - | 6.12 | 5.52 | | 6.12 | 5.52 | | |
| Critical Hdwy Stg 2 | | | | 2.218 | 9.5 | • | | | 2 240 | | 4.018 | 2.240 | |
| Follow-up Hdwy | 2.218 | | - | 888 | _ | _ | 3.518 129 | 4.018 153 | 3.318 | 3.518 | 156 | 3.318 | |
| Pot Cap-1 Maneuver | | | - | | - | - | | | | | | 600 | |
| Stage 1 | _ | - | | - | - | - | 393 | 410 | | 528 | 523 | | |
| Stage 2 | | - | - | - | • | - | 506 | 506 | - | 380 | 403 | | |
| Platoon blocked, % | 4074 | | | 000 | | - | 444 | 444 | 444 | 444 | 444 | 000 | |
| Mov Cap-1 Maneuver | 1071 | - | • | 888 | | | 114 | 141 | 441 | 111 | 144 | 600 | |
| Mov Cap-2 Maneuver | - | _ | | | | | 114 | 141 | _ | 111 | 144 | | |
| Stage 1 | - | • | - | | | - | 379 | 396 | • | 510 | 501 | | |
| Stage 2 | - | | | - | | | 457 | 484 | | 334 | 389 | | |
| Annanak | ED | | | 1AID | | | NID | - | | 00 | | | |
| Approach | EB | | | WB | | | NB | | | SB | a de la constante | | |
| HCM Control Delay, s | 0.4 | | | 0.7 | | | 32.1 | | | 23.6 | | | |
| HCM LOS | | | | | | | D | | | С | | | |
| At a discount distance of | | NAC - | EDI | | EDC | 16/892 | 1 A MOTO | MDD | 001 - 1 | £v." | . 3 | ~ 4 | b to the same that the |
| Minor Lane/Major Mvn | 1() | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | | F 2.1 | and the sales | 1994 - 1800 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | |
| Capacity (veh/h) | | | 1071 | - | - | 888 | | | 242 | | | | |
| HCM Lane V/C Ratio | | | 0.035 | - | | 0.043 | - | | 0.201 | | | | |
| HCM Control Delay (s) | | 32.1 | 8.5 | | - | 9.2 | ÷ | | 23.6 | | | | 1 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 |
| HCM Lane LOS | | D | Α | - | - | Α | - | - | С | | | | |
| HCM 95th %tile Q(veh | 1 | 1.4 | 0.1 | - | | 0.1 | - | | 0.7 | | | | |



Attachment 3

Analysis Worksheets for Cumulative Conditions

| Intersection | | | 1 | | | | | | | | | | |
|------------------------|--------|-------|------|--------|------|-----------|--------|-------|-------|--------|-------|-------|--|
| nt Delay, s/veh | 7.2 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | 7 | 1 | 7 | 4 | 1 | 7 | | 4 | | | 4 | | |
| Traffic Vol, veh/h | 19 | 339 | 20 | 48 | 571 | 16 | 32 | 1 | 51 | 40 | 2 | 46 | |
| Future Vol, veh/h | 19 | 339 | 20 | 48 | 571 | 16 | 32 | 1 | 51 | 40 | 2 | 46 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop | |
| RT Channelized | | U 1 | None | | | None | | 27 | None | | | None | |
| Storage Length | 415 | - | 415 | 415 | - | 415 | - | - | - | - | - | - | |
| Veh in Median Storage | e,# - | 0 | | - | 0 | | - | 0 | | - | 0 | -60 | |
| Grade, % | - | 0 | | | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mymt Flow | 21 | 373 | 22 | 55 | 656 | 18 | 46 | 1 | 74 | 56 | 3 | 64 | |
| | | | | | | | | | | | | | |
| Major/Minor | Major1 | | | Major2 | | | Minor1 | 1 | | Minor2 | - 17 | | |
| Conflicting Flow All | 674 | 0 | 0 | 395 | 0 | 0 | 1224 | 1199 | 373 | 1230 | 1203 | 656 | and a conservation of the second section and a second section and |
| Stage 1 | | | | - 1 | | 1102 | 415 | 415 | | 766 | 766 | - | |
| Stage 2 | | - | | _ | | | 809 | 784 | | 464 | 437 | | |
| Critical Hdwy | 4.12 | | | 4.12 | - 17 | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | |
| Critical Hdwy Stg 1 | 7.12 | | | - | | | 6.12 | 5.52 | - | 6.12 | 5.52 | - | |
| Critical Hdwy Stg 2 | | | | | | | 6.12 | 5.52 | | 6.12 | 5.52 | | |
| Follow-up Hdwy | 2.218 | - | | 2.218 | | | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 | |
| Pot Cap-1 Maneuver | 917 | | | 1164 | - | | 156 | 185 | 673 | 154 | 184 | 465 | |
| Stage 1 | - | | | - | | | 615 | 592 | - | 395 | 412 | | |
| Stage 2 | | - | | | | | 374 | 404 | | 578 | 579 | | |
| Platoon blocked, % | | _ | | | | | 0/1 | 101 | | 010 | 0,0 | | |
| Mov Cap-1 Maneuver | 917 | - 1 | | 1164 | | | 126 | 172 | 673 | 129 | 171 | 465 | |
| Mov Cap-2 Maneuver | - | | | - | | | 126 | 172 | - | 129 | 171 | ,00 | |
| Stage 1 | | | | | | 1 1/2 | 601 | 578 | | 386 | 393 | | |
| Stage 2 | | | _ | | | | 305 | 385 | | 501 | 566 | | |
| Olage 2 | | | | | | 17 | 000 | 000 | 471 | 001 | 500 | | |
| Approach | EB | | | WB | ELC. | 11. (1.4) | NB | | 75.5 | SB | 1100 | | and the same |
| HCM Control Delay, s | 0.5 | | | 0.6 | | | 32.3 | | | 44 | | | |
| HCM LOS | 0.0 | | | 0.0 | | | D | | | E | | | |
| TION LOS | | | | | | | J | | | | | | |
| Minor Lane/Major Mvr | nt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR. | SBLn1 | | | | The state of the s |
| Capacity (veh/h) | | 250 | 917 | | | 1164 | | | 209 | | | | |
| HCM Lane V/C Ratio | | 0.487 | | | | 0.047 | | | 0.585 | | | | |
| HCM Control Delay (s |) | 32.3 | 9 | | | 8.2 | | | 44 | | | | |
| HCM Lane LOS | | D | A | | _ | A | | | E | | | | |
| LIGHT EURO EUO | | | / 1 | | | 11 | _ | _ | - | | | | |

| Intersection | | | | | | | | | | | | |
|--|-------|----------|----------|--------|----------|----------|--------|-------|---------|--------|-------|------------|
| Int Delay, s/veh | 4.3 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7 | ^ | 7 | T | ↑ | 7 | | 4 | | | 4 | |
| Traffic Vol, veh/h | 34 | 660 | 14 | 50 | 434 | 41 | 23 | 2 | 37 | 13 | 1 | 25 |
| Future Vol, veh/h | 34 | 660 | 14 | 50 | 434 | 41 | 23 | 2 | 37 | 13 | 1 | 25 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | | | None | | | None | | | 44 | | | None |
| Storage Length | 415 | - | 415 | 415 | | 415 | | | | - | - | |
| Veh in Median Storage, | | 0 | | | 0 | | - | 0 | | | 0 | |
| Grade, % | | 0 | - | - | 0 | | - | 0 | | | 0 | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 37 | 725 | 15 | 57 | 499 | 47 | 33 | 3 | 54 | 18 | 1 | 35 |
| | | | | | | | | | | | | |
| Major/Minor M | ajor1 | | 1 | Major2 | | 1 | Minor1 | | 1 | Vinor2 | | |
| Conflicting Flow All | 546 | 0 | 0 | 740 | 0 | 0 | 1454 | 1459 | 725 | 1448 | 1427 | 499 |
| Stage 1 | - | | | | - | | 799 | 799 | - | 613 | 613 | |
| Stage 2 | - | | | | | | 655 | 660 | | 835 | 814 | |
| Critical Hdwy | 4.12 | 937. | | 4.12 | 772 | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | | | | | | | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | | | v | 1 . | | | 6.12 | 5.52 | | 6.12 | 5.52 | |
| | 2.218 | | | 2.218 | | | 3.518 | | 3.318 | 3.518 | 4.018 | 3.318 |
| | 1023 | | 3/2 | 867 | | | 108 | 129 | 425 | 109 | 135 | 572 |
| Stage 1 | - | | | | | - | 379 | 398 | - | 480 | 483 | |
| Stage 2 | - | | - | | 1772 | | 455 | 460 | | 362 | 391 | |
| Platoon blocked, % | | | | | | | | | | | | |
| | 1023 | - | 1 | 867 | - | | 93 | 116 | 425 | 86 | 122 | 572 |
| Mov Cap-2 Maneuver | - | | - | | | - | 93 | 116 | - | 86 | 122 | |
| Stage 1 | | | | 1 | | | 365 | 384 | | 463 | 451 | - |
| Stage 2 | - | | - | | - | | 398 | 430 | - | 303 | 377 | - |
| | | | | | | | | | | | | |
| Approach | EB | | | WB | | t | NB | ,(13) | £ - | SB | 51 | W. |
| HCM Control Delay, s | 0.4 | | 6 | 0.9 | | | 45 | | -1177 | 31 | | |
| HCM LOS | | | | | | | E | | | D | | |
| | | | | | | | | | | | | |
| Minor Lane/Major Mymt | | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR | SBLn1 | J. | | The second |
| Capacity (veh/h) | | 176 | | | | 867 | | | 192 | | | |
| HCM Lane V/C Ratio | | 0.511 | | | | 0.066 | | | 0.282 | | | |
| | | | | | | | | | | | | |
| The second secon | | 45 | 8.7 | | - | 9.4 | - | - | 31 | | | |
| HCM Control Delay (s) HCM Lane LOS | | 45 E | 8.7 A | 9-1- | | 9.4 A | | | 31 D | | | |



Attachment 4

Analysis Worksheets for Cumulative plus Project Conditions

| Intersection | in the | | | | | | | | | | | | |
|------------------------|--------|-----------------------|-------|--------|----------|----------|--------|-----------|--------|------------|------------|--------|------|
| Int Delay, s/veh | 7.3 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | 7 | 1 | 7 | 7 | † | 7 | 1100 | 4 | | | 4 | | |
| Traffic Vol, veh/h | 19 | 342 | 20 | 48 | 578 | 16 | 32 | 1 | 51 | 40 | 2 | 46 | |
| Future Vol, veh/h | 19 | 342 | 20 | 48 | 578 | 16 | 32 | 1 | 51 | 40 | 2 | 46 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop | |
| RT Channelized | 1100 | - | None | 1100 | - | None | Otop | - Otop | None | Otop | - Otop | None | |
| Storage Length | 415 | | 415 | 415 | | 415 | | | HOUG | | | 140110 | |
| Veh in Median Storage | | 0 | 413 | 413 | 0 | 413 | | 0 | - | | 0 | | |
| Grade, % | | 0 | | - | 0 | | | 0 | | | 0 | | |
| Peak Hour Factor | 91 | 91 | 91 | 87 | 87 | 87 | 69 | 69 | 69 | 72 | 72 | 72 | |
| | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Heavy Vehicles, % | 21 | 376 | 22 | 55 | 664 | 18 | 46 | 1 | 74 | 56 | 3 | 64 | |
| Mvmt Flow | 21 | 3/0 | 22 | 55 | 004 | 10 | 40 | - 1 | 14 | 30 | 3 | 04 | - |
| Major/Minor N | lajor1 | | | Major2 | | 1 | Minor1 | 4.5-55 | 1 | Minor2 | 10.0 | | |
| | 682 | 0 | 0 | 398 | 0 | | 1235 | 1210 | 376 | 1241 | 1214 | 664 | T. L |
| Conflicting Flow All | | | U | | | 0 | | | | | | | |
| Stage 1 | | | | | | | 418 | 418 | | 774 | 774 440 | | |
| Stage 2 | 4.40 | | | 4.40 | | | 817 | 792 | | 467 | | 6 22 | |
| Critical Hdwy | 4.12 | - | | 4.12 | | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | |
| Critical Hdwy Stg 1 | | _ | - | - | - | | 6.12 | 5.52 | - | 6.12 | 5.52 | | |
| Critical Hdwy Stg 2 | 0.040 | | | 0.040 | | | 6.12 | 5.52 | 2.240 | 6.12 | 5.52 | 2 240 | |
| | 2.218 | | - | 2.218 | - | | 3.518 | | | 3.518 | 4.018 | 3.318 | |
| Pot Cap-1 Maneuver | 911 | | - | 1161 | | | 153 | 183 | 670 | 152 391 | 182 408 | 461 | |
| Stage 1 | - | _ | | _ | _ | | 612 | 591 | - | | | | |
| Stage 2 | - | | • | | | | 370 | 401 | | 576 | 578 | • | |
| Platoon blocked, % | 044 | - | - | 4404 | | | 400 | 470 | 070 | 407 | 400 | 404 | |
| Mov Cap-1 Maneuver | 911 | | • | 1161 | | | 123 | 170 | 670 | 127 | 169 | 461 | |
| Mov Cap-2 Maneuver | - | - | - | - | | | 123 | 170 | | 127 | 169 | | , |
| Stage 1 | | - | | | • | - | 598 | 577 | | 382 | 389 | | |
| Stage 2 | | | | | - 31 | to - | 301 | 382 | - | 499 | 565 | 113 | |
| Approach | EB | - | | WB | | 156000 | NB | S (5) (6) | | SB | - | | |
| | | | | | | ll barri | | | | | | | |
| HCM Control Delay, s | 0.5 | | | 0.6 | | | 33.4 | | | 45.2 | | 912 | |
| HCM LOS | | | | | | | D | | | Е | | | |
| Minarl ana/Major Mum | | NBLn1 | EBL | EDT | EDD | WDI | WBT | WDD | 201 n4 | | | | |
| Minor Lane/Major Mym | | and the second second | - | EBT | EBR | WBL | | | SBLn1 | | | 1 | |
| Capacity (veh/h) | | 245 | 911 | - | | 1161 | | | 206 | | | | |
| HCM Lane V/C Ratio | | | 0.023 | - | | 0.048 | | | 0.593 | | | | |
| HCM Control Delay (s) | 1,37 | 33.4 | 9 | | | 8.3 | | 1: | 45.2 | | | 18 | |
| HCM Lane LOS | | D | | | | Α | | | E | | | | |
| HCM 95th %tile Q(veh) |) | 2.5 | 0.1 | - | - | 0.1 | | | 3.3 | | | | |

| 34 6 34 6 0 ree Fi - 415 - 91 2 | ree Fre - Non - 41 0 0 91 9 | 4 50 4 50 0 0 0 e Free e - 5 415 1 87 2 2 | WBT 438 438 0 Free - 0 0 87 2 503 | WBR 41 41 0 Free None 415 - 87 2 47 | 23 23 0 Stop - - - - 69 2 | NBT 2 2 0 Stop 0 0 69 | NBR 37 37 0 Stop None - - - 69 | 13 13 0 Stop | \$BT | SBR 25 25 0 Stop None | | |
|--|---|--|---|--|--|--|---|--------------------------------|-----------------------------|-------------------------------|--|---------------------------|
| 34 6 34 6 0 ree Fi -415 - -2 37 7 | 668 1.668 1.00 | 4 50 4 50 0 0 0 e Free e - 5 415 1 87 2 2 | 438 438 0 Free - 0 0 87 2 | 41 41 0 Free None 415 - 87 2 | 23 23 0 Stop - - - - 69 2 | 2 2 0 Stop - 0 0 69 | 37 37 0 Stop None | 13 13 0 Stop | 1 1 0 Stop | 25 25 0 Stop None | | |
| 34 6 34 6 0 0 ree Fi -415 - 91 2 37 7 | 668 1. 668 1. 0 ree Fre - Non- - 41 0 0 91 9 2 | 4 50 4 50 0 0 e Free e - 5 415 1 87 2 2 | 438 438 0 Free - 0 0 87 2 | 41 41 0 Free None 415 - 87 2 | 23 0 Stop - - - 69 2 | 2 2 0 Stop - 0 0 0 | 37 0 Stop None | 13 0 Stop - - - | 1 0 Stop - 0 | 25 0 Stop None | | |
| 34 6 0 ree Fi -415 - - 91 2 37 7 | 668 1. 0 ree Fre - Non - 41 0 0 91 9 | 4 50 0 0 e Free e - 5 415 1 87 2 2 | 438 0 Free - 0 0 87 2 | 41 0 Free None 415 - - 87 2 | 23 0 Stop - - - 69 2 | 2 2 0 Stop - 0 0 0 | 37 0 Stop None | 13 0 Stop - - - | 1 0 Stop - 0 | 25 0 Stop None | | |
| 0 ree Fi - 415 - 91 2 37 7 | 0 ree Fre - Non- 41 0 0 91 9 2 | 0 0 e Free e - 5 415 1 87 2 2 | 0 Free - 0 0 87 2 | 0 Free None 415 - - 87 2 | 0 Stop - - - - - 69 2 | 0 Stop - - 0 0 69 | O Stop None | O Stop | O Stop | O Stop None | | |
| ree Fi -415 | ree Fre - Non - 41 0 0 91 91 | e Free e - 5 415 1 87 2 2 | Free - 0 0 87 2 | Free None 415 - 87 2 | Stop - - - - 69 2 | Stop - - 0 0 0 69 | Stop None | Stop - - - | Stop - - 0 | Stop None | | |
| 91 2 37 7 | - Non- - 41 0 0 91 9 2 | 9 - 5 415 1 87 2 2 | 0 0 87 2 | None 415 - - 87 2 | 69 | 0 0 0 69 | None - - | | 0 | None - | | |
| 91 2 37 7 | - 41 0 0 91 9 2 | 5 415 1 87 2 2 | 0 0 87 2 | None 415 - - 87 2 | 69 | 0 0 0 69 | None - - | | 0 | None - | | |
| 91 2 37 7 | 0 0 91 9 2 | 1 87 2 2 | 0 0 87 2 | - 87 2 | 69 2 | 0 0 69 | 100 | - | - | | | |
| 91 2 37 7 | 0 0 91 9 2 | 1 87 2 2 | 0 87 2 | - 87 2 | 69 2 | 0 69 | | - | - | | | |
| 91 2 37 7 | 91 9 | 1 87 2 2 | 87 2 | 87 2 | 69 2 | 69 | | | - | - | | |
| 2 37 7 or1 | 2 | 2 2 | 87 2 | 2 | 2 | | 60 | | | | | |
| 2 37 7 or1 | 2 | 2 2 | 2 | 2 | 2 | | 09 | 72 | 72 | 72 | | |
| 37 7 | | | | | | 2 | 2 | 2 | 2 | 2 | | |
| or1 | | - | - | | 27.7 | 3 | 54 | 18 | 1 | 35 | | |
| | 1111111111111111111111111111111 | | | | | | | 10 | | - 00 | | |
| | | Major2 | | 1 | Minor1 | 15-17- | | Minor2 | | 2 10 1 | The same of the sa | W 25 1 4 1 |
| | 0 | 749 | 0 | 0 | 1467 | 1472 | 734 | 1461 | 1440 | 503 | and are the land of the same | and the first way |
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| 12 | i i | _ // 12 | | 355 | | | | | | 622 | | |
| | | 7.12 | _ | _ | | | | | | 0.22 | | |
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| | | - 2 218 | | | | | | | | | | |
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| - | | | | _ | 400 | 400 | • | 330 | 300 | • | | |
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| - | · •/. | | - | - | - | | | | | - | | |
| | 91335 | | en ai | | 390 | 420 | 95.00 | 299 | 3/4 | | | |
| FR | | WR | | | NR | | | SR | 55.000 | | | |
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| U. 4 | | 0.9 | | | | | | | | | | |
| | | | | | | | | J | | | | |
| NBI | n1 EB | L EBT | EBR | WBL | WBT | WBR | SBLn1 | - | | - 53 | 2-3 | |
| | | | | | | _ | | | | ant to the | | £ 2 £ 2.400 - 2.02 × 0.00 |
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| 4 | | | | | | | | | | | | |
| | | | | | | | | | | | | |
| 200 | | | 112 - 4.12 | 112 - 4.12 | 12 | | | | 808 808 - 617 659 664 - 844 | | | |

ADDENDUM TO INITIAL STUDY/MITIGATED NEGATIVE DECLARATION A07-0005/Z07-0004/PD07-0007/TM07-1440/Summer Brook Estates

RECEIVED

MAR 2 0 2023

I. INTRODUCTION

EL DORADO COUNTY PLANNING AND BUILDING DEPARTMENT

A. DETERMINATION

This document constitutes an Addendum to the March 11, 2008, Initial Study/Mitigated Negative Declaration (2008 IS/MND) prepared for the Summer Brook Estates Tentative Subdivision Map (hereafter referred to as the Project). This Addendum evaluates whether an amendment to a condition of the Project (modification of Condition 25), consisting of the requirement to construct traffic signals at the intersection of Green Valley Road and Deer Valley Road (the "Study Intersection"), would result in any new or substantially more adverse significant effects or require any new mitigation measures not identified in the 2008 IS/MND and otherwise be consistent with the intent of the County in approving the Project. Other than the elimination of the traffic signals and installation of intersection lighting, as discussed below, there are no other modifications to the Project as approved.

Accordingly, all other aspects of the analyses and the conclusions in the 2008 IS/MND remain current and valid. The proposed minor modification to condition #25 would not cause new impacts not already identified in the MND nor increase the level of environmental effect to substantial or significant, and, hence, no new mitigation measures would be necessary to reduce significant effects. No change has occurred with respect to circumstances surrounding the Project that would cause new or substantially more severe significant environmental effects than were identified in the 2008 IS/MND. Additionally, no new information has become available that shows that the Project would cause new or substantially more severe significant environmental effects which have not already been analyzed in the 2008 IS/MND. There is, however, new information and circumstances that reveal that the effects of the Project are far less than discussed in the 2008 IS/MND. Therefore, no further environmental review is required beyond this Addendum.

B. BACKGROUND

The Project was formally evaluated in a 2008 IS/MND for the Summer Brook Estates Tentative Subdivision Map which was approved on March 11, 2008. The 2008 IS/MND was prepared pursuant to the California Environmental Quality Act (CEQA) and adopted by the County. The Project as approved consists of 29 residential lots ranging in size from 58,591 to 91,113 square feet. The tentative map for the Project was extended in 2017; a phasing plan for the Project was approved in 2018; and a final map for the first two phases of the Project were approved by the Board of Supervisors on October 11, 2022.

In support of the approval of the Project in 2008, a traffic study was performed which revealed, based on then available information, that the Study Intersection was in a failing condition, with respect to level of service, and met the warrants for signalization and that the

addition of traffic from the Project and other approved and pending projects would exacerbate the condition and thus constitute a significant impact and violate the General Plan policies relative to roadway levels of service. Accordingly, conditions of approval were imposed on the Project (Conditions 25, 27 and 28) which describe improvements to the intersection including significant widening, addition of turn and acceleration and deceleration lanes and signalization ("Intersection Improvements").

Since the approval of the Project there have been significant changes in circumstance surrounding the Project and the Study Intersection. First and foremost, the original traffic study which identified the need for the improvements was based on assumptions and a traffic model which have been proven to be inaccurate, overstating the project traffic in the area. Further, since the adoption of the 2008 IS/MND major improvements have been made to the Study Intersection in accordance with the conditions of approval of the Silver Springs Subdivision at the Green Valley Road and Silver Springs Parkway intersection. In addition to the intersection improvements at the Study Intersection, the traffic signals at the Green Valley Road/Silver Springs Parkway intersection have been installed and energized which have had the effect of breaking up traffic and diverting much of the traffic from Green Valley Road. The result of the adoption of the updated traffic demand model and the construction of these improvements is that the conditions which were existing and projected at the intersection as presented in the 2008 IS/MND no longer exist. The intersection operates in accordance with the General Plan traffic standards and does not meet warrants for the installation of a traffic signal.

C. PURPOSE OF THIS ADDENDUM

In light of the changes in circumstance, as discussed above, the County Department of Transportation has determined that a traffic signal is no longer warranted, and the Project applicant has proposed the installation of LED intersection lighting in accordance with County standards. The Project will be responsible for the cost of electricity and maintenance of the lighting thus imposing no burdens on the County. The purpose of this Addendum is to evaluate whether a traffic signal at the intersection is needed for mitigation of the traffic impacts of the Project and would the modification of the project conditions eliminating the traffic signal and requiring the installation of pole lighting at the intersection result in any new or substantially greater significant effects or require any new mitigation measures not identified in the 2008 IS/MND for the Project. This Addendum, together with the 2008 IS/MND will be used by the County as the environmental clearance for the modification of the Project condition.

D. CEQA FRAMEWORK FOR ADDENDUM

For a proposed modified project, State CEQA Guidelines (Sections 15162 and 15164) provide that an Addendum to an adopted MND may be prepared if only minor technical changes or additions are necessary or none of the following conditions calling for the preparation of a subsequent MND have occurred:

Substantial changes in the project which require major revisions to the MND due to the involvement of new significant environmental effects or a substantial increase in the severity of previously identified significant effects; or

Substantial changes with respect to the circumstances under which the project is undertaken which require major revisions to the MND due to the involvement of new significant environmental effects or a substantial increase in the severity of previously identified significant effects; or

New information of substantial importance, which was not known and could not have been known with the exercise of reasonable diligence at the time of MND adoption, shows any of the following:

- i) the project will have one or more significant effects not discussed in the MND.
- ii) the project will result in impacts substantially more severe than those disclosed in the MND,
- iii) mitigation measures or alternatives previously found not to be feasible would in fact be feasible and would substantially reduce one or more significant effects of the project, but the project proponent declines to adopt the mitigation measure or alternative, or
- iv) mitigation measures or alternatives that are considerably different from those analyzed in the MND would substantially reduce one or more significant effects on the environment, but the project proponent declines to adopt the mitigation measure or alternative.

The purpose of this Addendum is to evaluate the elimination of the traffic signal as called for in the 2008 IS/MND for the Project. Based on the analysis provided below, an Addendum to the 2008 IS/MND is the appropriate CEQA document.

E. DISCUSSION

This Addendum can be very narrowly focused on its discussion as the only modification to the Project is the elimination of the traffic signals and installation of pole lights at the intersection. In considering the CEQA checklist for potential impacts the only relevant inquiries would be traffic impacts and aesthetic impacts. The elimination of the traffic signals will have no appreciable impact, or any impact different than outlined in the 2008 IS/MND on biological resources, greenhouse gas emissions, land use/planning/ population/housing, noise, agriculture; cultural resources, mineral resources, public services, utilities, air quality, geology/soils, hydrology/water quality, or recreation. Accordingly, these topics are not discussed.

Aesthetics considerations are raised in response to concern that the installation of pole lighting may have visual effects when viewed from adjacent properties, particularly at night with the introduction of light in an otherwise dark area. With respect to physical appearance of light standards and poles the appearance will not be dissimilar to the poles and standards associated with traffic signals. In fact, the lamp heads associated with LED fixtures will be smaller and less obtrusive in appearance than the mast heads for traffic signals. The biggest difference in appearance will occur in the nighttime hours, where the pole lamps will provide a constant

source of white light during the entire period of darkness, whereas the traffic signals will provide a changing light pattern associated with the red, yellow and green traffic control lights. Both pole lighting and traffic signals utilize shielding to minimize side casting light creating minimal impacts to surrounding areas. There will be little or no appreciable difference in impacts. Accordingly, it is concluded that the replacement of the traffic signals with LED light poles will not result in a new significant aesthetic impact.

To evaluate the potential the traffic impact of the elimination of the traffic signals at the Study Intersection, a supplemental traffic study and signal warrants analysis was performed by Kimley-Horn, and presented in a memorandum dated November 21, 2022, attached to this Addendum as Exhibit A (the "Supplement Study"). Kimley-Horn did the original traffic analysis in 2007, which was referenced in the 2008 IS/MND and they performed subsequent assessments of the intersection in 2019, 2021 and 2022. These additional studies were performed in response to multiple factors, very notably the fact that the County had updated its Traffic Demand Model since the 2007 study was performed, which revealed far lower projected traffic volumes than originally were assumed. This was the result of population growth assumptions and projections that had been grossly overstated in earlier studies. As a result, the Supplemental Study reveals that AM peak hour traffic volumes measured in 2021 and 2022 were actually lower than the baseline volumes assumed in 2007. Further, major improvements at the Study Intersection were performed including turning and acceleration and deceleration lanes in both directions on Green Valley Road (Conditions of Approval 27 and 28 are satisfied by these improvements). With these improvements the traffic flow through the intersection is significantly improved. The Supplemental Study concludes that with the improvements and the updated traffic information, that the intersection operates within the General Plan standards in both current conditions and projected future conditions. Further, the traffic signal warrants evaluations done with the updated information dated March 28, 2019, and June 4, 2021, concluded that a traffic signal was not warranted for both the No Project and Plus Project scenarios at the study intersection.

The importance of the conclusion that the intersection does not meet warrants is that it is acknowledged by traffic engineers and transportation planners that traffic signals should not be utilized in situations where warrants are not met. Unwarranted signalization disrupts traffic flow and may actually negatively impact the safe operation of the intersection. According to the Caltrans Local Road Safety Manual, lighting an intersection, as being proposed, not currently lit has a crash modification factor of forty percent (40%). In other words, lighting an intersection could reduce nighttime crashes by upwards of forty percent (40%). Although County records currently show that there have been no crashes at the intersection since the improvements, discussed above, were constructed, the substitution of traffic signals for pole lighting at the intersection will enhance the safety of the intersection.

In summary, the Supplemental Study concludes that due to the update provided for the County's Transportation Demand Model and the subsequent physical improvements to the Study Intersection, traffic signals at the intersection are not warranted or desirable. The revised condition provides for a lighted intersection in lieu of the traffic signal does not present any new or otherwise unrevealed impacts. Further, the reduced traffic projections for Green Valley Road incrementally reduce any potential impacts. The updated analysis does not identify any new or modified mitigation requirements for traffic.

F. CONCLUSION

On the basis of the discussion above, the modification of Condition of Approval #25 originally called for in the Project, will not trigger any conditions requiring further CEQA review. Thus, this Addendum satisfies the requirements of CEQA Guidelines 15162 and 15164. The modification to the Project does not introduce new significant environmental effects, substantially increase the severity of previously identified environmental effects, or show that mitigation measures or alternatives previously found not to be feasible would in fact be feasible. The analyses and conclusions in the 2008 IS/MSD remain current and valid.